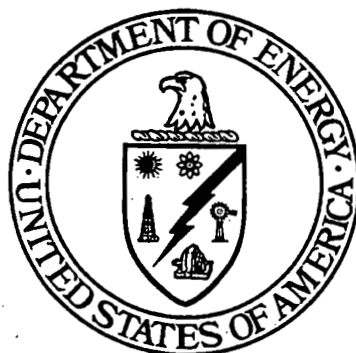


**AREA 9, PHASE III
ABANDONED OUTFALL LINE
EXCAVATION PLAN
PART TWO**

**FERNALD CLOSURE PROJECT
FERNALD, OHIO**



SEPTEMBER 2004

U.S. DEPARTMENT OF ENERGY

**21140-PL-0003
REVISION A
DRAFT**

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LIST OF ACRONYMS AND ABBREVIATIONS

CM	Construction Manager
D&D	Decontamination and Decommission
DOE	U.S. Department of Energy
FCP	Fernald Closure Project
FRL	Final Remediation Level
NPDES	National Pollutant Discharge Elimination System
OSDF	On-Site Disposal Facility
WAC	Waste Acceptance Criteria
WAO	Waste Acceptance Organization

1.0 INTRODUCTION

1.1 BACKGROUND

In the early 1950's, construction began on the Feed Materials Production Center in Hamilton and Butler Counties in southwestern Ohio to convert uranium ore into uranium metal and to fabricate uranium metal into target elements for reactors that produced plutonium and tritium. In 1951, a 16-inch cast iron outfall sewer line was installed from the Feed Materials Production Center to the Great Miami River (see Figure 1-1). There were two wastewater streams that were discharged through the 16-inch cast iron outfall sewer line. One was a treated effluent wastewater stream from the production area that was carried through a 12-inch treated effluent line. The other was a treated sanitary wastewater stream from the site's Sewage Treatment Plant that was carried through a 10-inch treated sanitary sewer line. Both wastewater streams entered the 16-inch cast iron outfall sewer line at manhole MH 175 near the eastern edge of the Fernald Closure Project (FCP) site.

The 16-inch cast iron outfall sewer line was designed partially as a pressurized system. Originally, there were six manholes installed off-property (MH's 177, 178, 179, 180, 181, and 182). MHs 178 through 182 were installed as pressurized manholes made of reinforced concrete vaults with gaskets and bolted pressure-tight manhole lids and rings. MH 175, 176, and 177 were installed as non-pressurized manholes.

Originally Manhole MH182 was placed approximately 140 feet from the discharge end of the pipeline west of the riverbank; however due to strong erosion, the west riverbank of the Great Miami River had eroded so that during a flood in 1965 MH182 toppled into the river and the pipeline was severed so that it no longer discharged into the main channel. It is probable that sections of the pipeline near the end became detached as the riverbank eroded westward between the early 1950's and 1965.

A construction project was started in 1966 to rebuild the outfall. The pipeline was encased in sheet piling, bank run gravel and concrete. The discharge point was approximately 30 feet out from the bottom of the west riverbank and 90 feet from the top of the west riverbank. By 1968, the riverbank was becoming badly eroded again. Broken concrete was dumped along the riverbank in the general vicinity of the outfall structure. This concrete was obtained from a construction project on North Bend Road in Hamilton County and was not obtained from the FCP.

By 1973, floodwaters had eroded the riverbed around the outfall structure such that approximately 20 feet of sheet piling that extended out into the river was washed out and the end of the pipeline was severed. As such, part of the outfall structure was rebuilt by replacing some of the existing sheet piling in the river,

raising and resetting the sewer pipe in crushed stone, and installing riprap to protect the outfall structure from erosion. Crossties were installed at the top of the rebuilt section that tied the sheet piling on both sides of the pipeline together. Contaminated concrete from the FCP was also placed in the river to protect the rebuilt outfall structure and along the upper riverbank to prevent damage during periods of flooding.

In April 1989, Manhole 180 was observed to overflow storm water onto the surrounding ground. It was discovered during repairs to the manhole that no gasket existed under the manhole cover.

In the early 1990's, a 24-inch High Density Polyethylene (HDPE) outfall sewer line was installed from the Fernald Closure Project (FCP) (originally known as the Feeds Material Production Center) to the Great Miami River approximately 50 feet to the south of the original outfall line (see Figure 1-1).

At that time, the original outfall line was taken out of service and abandoned in place, henceforth referred to as the abandoned outfall line. The manholes designations for the abandoned outfall line were changed by adding an "S" at the beginning and an "A" at the end (e.g.: MH 181 became SMH 181A). The 24-inch HDPE outfall sewer line is currently active and discharging into the Great Miami River.

In 1999, the sewage treatment plant area was remediated. As part of this remediation, the on-property portion of the abandoned outfall line was removed from SMH 175A (formerly MH 175) to SMH 176A and placed in the On-Site Disposal Facility (OSDF).

1.2 PURPOSE

Remediation of the abandoned outfall line is included in Area 9 Phase III as shown in Figure 1-2. The remediation of the abandoned outfall line is planned in three parts. Part One includes the remediation of the abandoned outfall line from just inside the FCP property line approximately at Station 0+00 to just west of the State Route 128 right of way approximately at Station 21+00. This excludes approximately 85 feet of pipe and Manhole MH 177A between Station 2+32 and Station 3+17 located in the vicinity of the crude oil pipeline owned by the Mid Valley Pipeline Company.

Part Two includes the remediation of the abandoned outfall line at the discharge structure (also known as the outfall structure) along the west bank of the Great Miami River and the restoration of the river bank at the discharge structure. Part Three includes the remediation of the abandoned outfall line from Station 21+00 to the outfall structure approximately at Station 38+00, including the remediation of abandoned outfall line underneath State Route 128, and the remediation of the section of abandoned outfall line under the crude oil pipeline owned by the Mid Valley Pipeline Company.

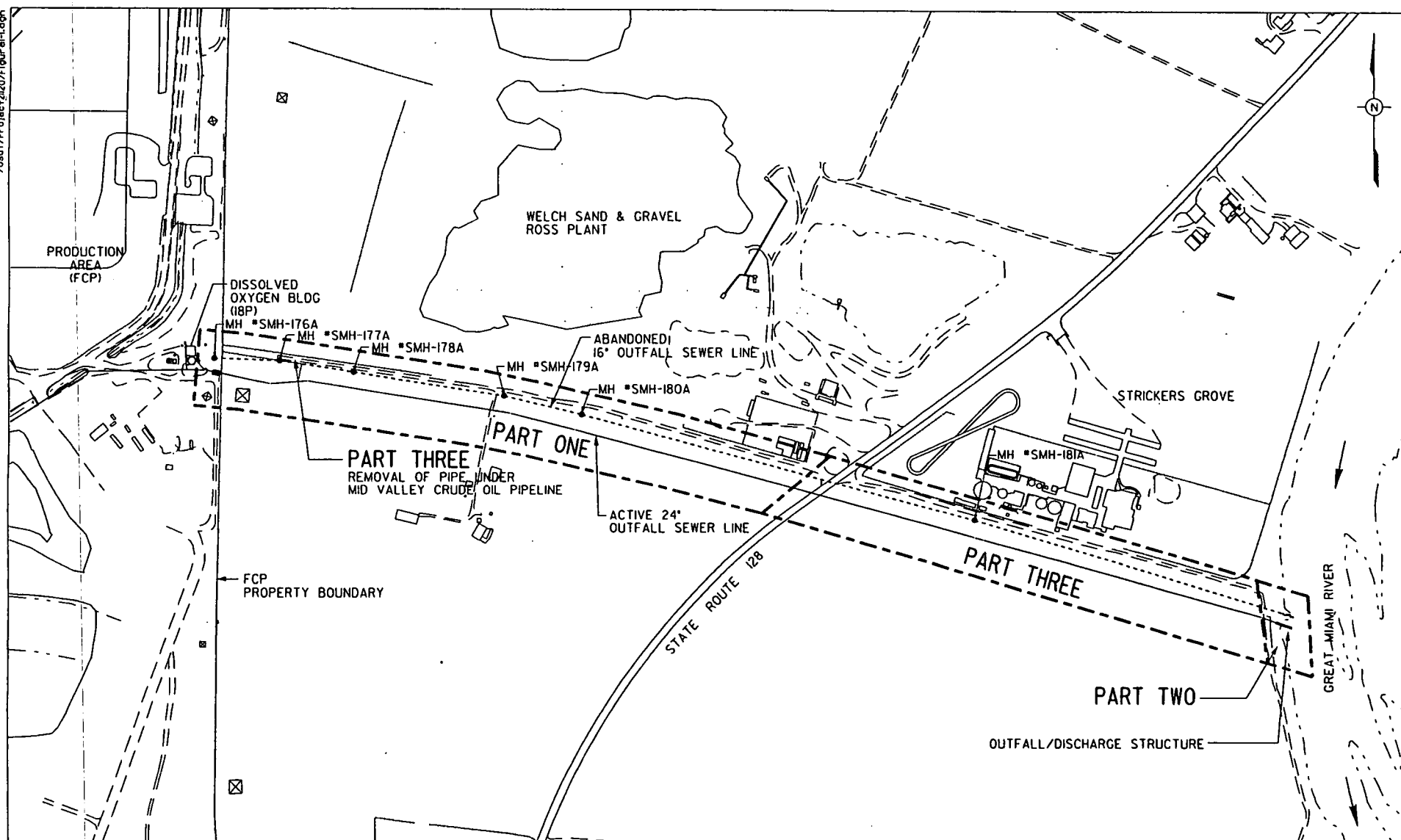
1 The Excavation Plan Part One, Rev. 0 submittal governed the remediation of Part One and was
2 transmitted to the agencies for review on August 19, 2004. The Excavation Plan Part Three submittal will
3 govern the Part One remediation of the abandoned outfall line and will be submitted to the agencies for
4 review at a later date. This document represents the Part Two Excavation Plan submittal and governs the
5 Part Two remediation of the abandoned outfall line.

6
7 The remedial actions addressed in this Part Two Excavation Plan include removing an estimated 50 bcy
8 (bank cubic yards) of impacted soil, 400 bcy of debris, and 140 linear feet of pipe for disposal as
9 discussed in Section 3.3. It is estimated that 0.6 cubic feet of lead will be removed from the pipe joints
10 and segregated for off-site disposal.

11
12 Most of this remedial action will take place off-property [not on the Fernald Closure Project (FCP)
13 property]. For purposes of this document, references to "on-site" will mean on the FCP property and
14 references to "off-site" will mean off the FCP property.

15 16 1.3 EXCLUSIONS

17 Active 24-inch HDPE outfall sewer line and a nearby 4-inch abandoned gas line are excluded from this
18 document. Potentially contaminated concrete located south of the active outfall structure is excluded
19 from this document.



AREA 9 PHASE III
REMEDATION OF ABANDONED OUTFALL LINE
EXCAVATION PLAN - PART TWO

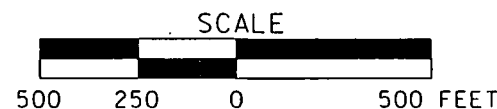


FIGURE I-1 SITE PLAN

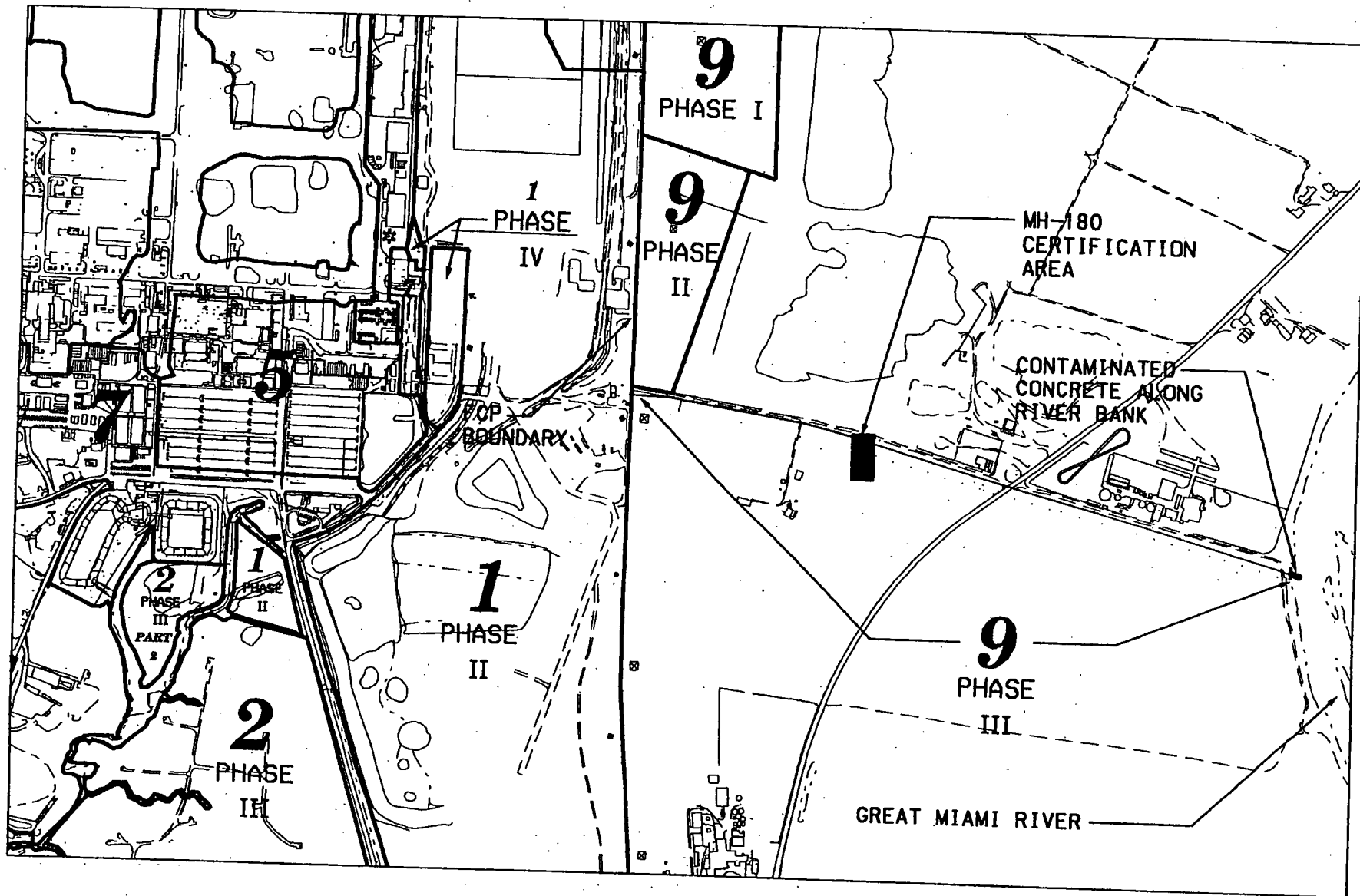


FIGURE 1-2. AREA 9 PHASE III INDEX MAP

2.0 PREDESIGN INVESTIGATION AND CHARACTERIZATION DATA

2.0 SUMMARY OF HISTORICAL AND PREDESIGN INVESTIGATIONS

2.1.1 Historical Investigations

In 1993, prior to the establishment of OU5 off-site soil FRLs, samples were collected along the Great Miami River bank near the point of the outfall line discharge under the Project Specific Plan for the Great Miami Riverbank Characterization, May 1993 and the Supplement to Project Specific Plan Great Miami Riverbank Characterization, December 1993 in response to the removal of contaminated concrete described in Removal of Contaminated Rubble Near the FEMP Wastewater Discharge in the Great Miami River, July 1992. The samples were split between an off-site commercial laboratory and the on-site laboratory. The on-site sample results, which were above the now established OU5 off-site soil FRLs in four borings, appear to be questionably high in comparison to the off-site laboratory's analytical results. All of the results from the off-site laboratory were below the OU5 off-site soil FRLs. The historical data are presented in Appendix B, Tables B-1 and Figure B1-1 of this plan.

2.1.2 Predesign

The Project Specific Plan for Area 9, Phase III Outfall Ditch Predesign Investigation (21140-PSP-0002) has been completed and submitted to the agencies for review in May 2004. This predesign PSP directed the collection of confirmatory samples from the historical borings whose results were above the OU5 off-site soil FRLs. The predesign sampling data are reported in Appendix B, Table B-2 and Figure B2-1 of this plan.

2.2 ABOVE-FRL/IMPACTED MATERIAL EXCAVATION BOUNDARIES

All newly collected predesign samples did not confirm the presence of above-FRL material and therefore demonstrate that there is no soil contamination. Impacted material removal boundaries have been established at the extent of the historical sampling performed after the original Removal of Contaminated Rubble Near the FEMP Wastewater Discharge in the Great Miami River was completed in 1992. As discussed in Section 2.1.1 above, historical sampling was performed in 1993 under the Project Specific Plan for the Great Miami Riverbank Characterization and the Supplement to Project Specific Plan Great Miami Riverbank Characterization. The sample results collected from borings 11172, 11178, 11179, and 11181 coupled with information presented in Removal of Contaminated Rubble Near the FEMP Wastewater Discharge in the Great Miami River resulted in the establishment of the excavation boundaries (see Section 3.3.2).

3.0 REMEDIAL APPROACH

3.1 INTRODUCTION

Part Two remediation of the abandoned outfall line will consist of four primary activities: site preparation, remedial excavation, certification, and restoration.

3.2 SITE PREPARATION

Site preparation activities associated with this work include but are not limited to: protection of utilities, establishing site boundaries and controls, surveying, site layout, construction fencing and signage, erosion and sediment controls, storm water and construction water management systems, and establishing support facilities, site access and haul route. Site preparation activities are scheduled to begin early fall 2004.

Detail description for site preparation activities are provided in the following text and in referenced design drawings and technical specifications.

3.2.1 Protection of Existing Utilities

There are a number of utilities located along and across the haul road for Part Two remediation of the abandoned outfall line. Underground utilities within or near the work area will be surveyed and marked in the field prior to the initiation of site preparation activities. These include the Cincinnati Gas and Electric (CG&E) overhead transmission lines, CG&E underground gas lines, Cincinnati Water Works (CWW) water main, Cincinnati Bell telephone line, Mid Valley crude oil pipeline, and 24-inch HDPE Outfall Sewer Line.

3.2.1.1 CG&E Overhead Transmission and Power Lines

CG&E overhead transmission lines are located directly over a portion of the haul route at approximately Station 1+10 of the abandoned outfall line. In addition, overhead electrical power lines run parallel to both sides of State Route 128 and are positioned over a portion of the Part Two haul route. Equipment will be chosen so that safe working distances will be maintained between the operating height of the equipment and the CG&E overhead transmission lines and overhead electric power lines.

An old unoccupied farmhouse is located approximately 500 feet south of the abandoned outfall line at Station 11+00 and is outside the work area. An overhead electrical power line runs from State Route 128 to the farmhouse and parallels the haul route from State Route 128 to approximately Station 10+80. This electrical power line has been de-energized, but the wires were left in-place during Part One activities.

1 These wires may be re-energized during Part Two activities. Equipment will be chosen and work
2 practices followed so that safe working distances will be maintained between the operating height of the
3 equipment and this electrical power line.

5 3.2.1.2 CG&E Underground Gas Lines

6 An active CG&E underground gas line runs parallel to State Route 128 and intersects the abandoned
7 outfall line at approximately Station 21+80. It is very important to protect this gas line during the entire
8 duration of the remediation and restoration of the abandoned outfall line. The primary hazard to this
9 pipeline includes remediation work performed near and underneath the gas line. This line should already
10 have sufficient cover, as required for crossing of construction equipment allowed on State highways.
11 Approximately 120 feet northeast of the excavation location, haul trucks from the Welch's gravel pit
12 drive over the CG&E gas line many times each work day. As part of Part Two site preparation, the
13 entrance drive off of State Route 128 was upgraded over the CG&E gas main prior to the commencement
14 of construction activities in accordance with requirements of CG&E.

16 An abandoned 4-inch CG&E gas line that once provided natural gas to the FCP is located
17 approximately 35 to 40 feet south of the abandoned outfall line between the site property boundary and
18 State Route 128. A CG&E technician blocked this line by inserting a "paddy" between the flanges to
19 physically obstruct the flow to the 4-inch gas line. The line is physically isolated from the gas main that
20 runs parallel to State Route 128 in accordance with CG&E requirements.

22 3.2.1.3 Mid Valley Crude Oil Pipeline

23 At approximately Station 2+80, a crude oil pipeline owned by Mid Valley Pipeline Company crosses over
24 the alignment for the abandoned outfall line. This line is a 20-inch diameter welded steel pipe and is used
25 to transfer crude oil under pressure. It is critical that this line be protected during the entire duration of
26 the remediation and restoration of the abandoned outfall line.

28 The primary hazard to this pipeline during Part Two remediation is the hauling of construction and
29 impacted materials over the pipe. To eliminate or minimize this hazard, a protective covering was
30 constructed over this pipeline as part of the site preparation for Part One of the abandoned outfall line in
31 accordance with Mid Valley Pipeline Company requirements.

3.2.1.4 CWW Water Main

An active Cincinnati Water Works (CWW) underground water main runs parallel to State Route 128 and intersects the abandoned outfall line at approximately station 22+20. This water main will be protected during the entire duration of the remediation and restoration of the abandoned outfall line. The Part Two remediation of the abandoned outfall line does not include any excavation within the vicinity of the CWW water main.

3.2.2 Surveying

Prior to site preparation and excavation activities, the remediation area will be surveyed to establish temporary benchmarks, locate existing utilities, easements, and property boundaries, and layout construction fence, abandoned outfall line, centerline of haul road, access ramp, and excavation limits for contaminated broken concrete.

3.2.3 Construction Fencing

3.2.3.1 Construction Fence/Access Control

Construction safety fence will be installed around the work area prior to removal of topsoil in accordance with design drawings 99X-5500-G-00807 and 99X-5500-G-00799. The work area will be of sufficient size to allow for the work to be performed safely and stockpiling of topsoil and overburden to be placed within the fenced in area. Construction safety fence will be removed after completion of Part Two remediation, as needed.

An access control (posts with removable cable) was installed at the FCP property boundary as part of Part One site preparation activities. An existing gate on the gravel drive west of State Route 128 was utilized as part of Part One and will continue to be used for Parts Two and Three. Both the access control at the FCP property boundary and the existing gate at State Route 128 will be closed whenever project personnel are not present within the work area (e.g., during lunch breaks and at the end of shift). Rope gates along the haul road east of State Route 128 will be installed as necessary. Temporary openings will be made in the construction safety fence to provide access to the work area as needed.

The haul road east of State Route 128 will also be used to provide access to Stricker's Grove. Post's with removable cables will be installed in the construction fence to provide access to Stricker's Grove.

In addition, a rope gate will be installed near the riverbank to control access to the work area.

3.2.3.2 Work Area Entry Requirements

Whenever the access control at the FCP property boundary is opened, personnel within the off-site work area must wear a site access badge. This requirement will not apply during construction activities within the off-site work area provided the site boundary access control is secured.

3.2.4 Erosion and Sediment Controls

Prior to topsoil removal and before start of excavation, silt fence will be installed downstream of the stockpiling of topsoil and overburden material and as is practical to be between the above-water work area and the Great Miami River. Topsoil and overburden material will be stockpiled during excavation within the construction safety fence limits. See design drawing 99X-5500-G-00778 and technical specification section 02275 for silt fence detail and installation requirements. Silt fence will also be installed to control runoff from the on-site impacted material stockpile areas, as required.

3.2.5 Part Two Construction Water Systems

Construction water includes standing water in the abandoned outfall line, subsurface water, storm runoff, and effluent water within the excavation, excluding river water. Since most of the remedial activities for Part Two occur within the vicinity of the Great Miami River and the associated riverbank, the collection of construction water separate from river water will be very difficult. As a result, runoff from the riverbank and access ramp will generally be filtered through a silt fence prior to being released into the Great Miami River. In addition, some construction water may be collected within the excavation, primarily if the water level in the river is low, and will be contained and treated, if necessary. Construction water does not include water within the work area that is either at or below the actual water level of the Great Miami River. No attempt will be made to control or collect river water.

Rinsate generated from cleaning construction equipment coming in contact with the abandoned outfall line, associated bedding material, or impacted soil will be transported back to the FCP for on-site AWWT treatment.

3.2.6 Haul Road

A haul route will be established from the Part Two work area at the riverbank of the Great Miami River to either an on-site material transfer area for subsequent placement into the OSDF or the on-site SP-7 stockpile area for subsequent disposal off-site. The haul route from State Route 128 to the SP-7 stockpile area was established during Part One site preparation activities (see Section 3.2.6 of the Area 9, Phase III Abandoned Outfall Line Excavation Plan, Part One). During Part Two site preparation, an

existing off-site gravel drive from State Route 128 to the west riverbank of the Great Miami River will be improved. In addition, a ramp will be excavated into the riverbank to provide access to the outfall structure and riverbed.

The haul road to be used during the Part Two remediation of the abandoned outfall line will be operated as a "clean" haul route and a portion will go through a certified areas. To ensure the integrity of the certified areas are met, a controlled loading process will be utilized. This assures the load in the haul trucks, including roll off boxes, will be placed below the sideboards, and that side rails and tailgates will be scraped cleaned prior to hauling. If materials are spilled during the loading process it will be cleaned up prior to the trucks exiting the loading and dumping zones. Trucks must be visibly clean prior to exiting these buffer zones. Haul trucks will be equipped with load covers and employ Best Available Technology (BAT) methods to ensure that no airborne materials are emitted from the haul truck bed or load. Load covers will be extended to cover the truck beds, whether full or empty, during equipment movement. The dumping process will require that the trucks be staged in a clean buffer area and dump hauled materials into the contamination area via an elevated ramp or platform that prevents the dumped material from spilling back against the tires. Trucks will have to be scraped clean and wheel washed prior to leaving the clean buffer area and returning to the excavation site.

3.2.6.1 Gravel Road Improvement East of State Route 128

An approximately 10-foot wide gravel drive exists along the north side of the abandoned outfall line from State Route 128 to the west riverbank of the Great Miami River. This grave drive will be upgraded to function as part of the off-site haul road for Part Two. This portion of the off-site haul road will be widened to 15 feet and improved by placing 4 to 6 inches of angular American Association of State Highway and Transportation Officials (AASHTO) #57 or ODOT #617 aggregate over the 15-foot wide haul road subgrade. In places where an unsuitable subgrade is encountered, geotextile separator and/or AASHTO #2 stone will be placed to improve the subgrade prior to the placement of the angular AASHTO #57 or ODOT #617 aggregate.

The off-site haul road tie-in to the east side of State Route 128 will be improved to protect edge of highway pavement and the CWW water main. This will be done by placing AASHTO #57 or ODOT #617 limestone over the approach to State Route 128.

Truck turn arounds will be installed near the Great Miami River. In addition, an equipment and approximately at Station 27+30. Another truck turn around was installed west of State Route 128 during

1 site preparation for Part Two approximately at Station 10+80 and will remain in service through Part Two
2 and Three remediation.

3.2.6.2 Protection Over Mid Valley Pipeline

5 The protective cover over the Mid Valley crude oil pipeline that was constructed as part of the Part One
6 site preparation activities will be maintained in accordance with requirements from the
7 Mid Valley Pipeline Company and utilized during Part Two remediation.

3.2.6.3 On-Site Haul Road

10 Drawing 99X-5500-G-00797 shows the on-site haul route for direct haul of impacted material from the
11 off-site work area to the on-site SP7 stockpile area.

3.2.7 Off-Site Backfill Sources

14 Backfill and topsoil material, if required, will be borrowed from off-site sources. Sand may be used as
15 supplemental general fill material and may be obtained from nearby sand and gravel pits. Topsoil may
16 also be borrowed from an off-site source with input from the property owner.

3.2.8 Pre-Excavation Sampling

19 Prior to the initiation of excavation, sediment/residue samples were taken from the bottom of the
20 abandoned outfall line manholes. These residues were analyzed for various constituents to determine if
21 pipe residues are likely to meet or exceed radiological and chemical WAC. The results of the residue
22 sampling demonstrated that the sediments in the manholes are above-WAC. Soil samples were also
23 collected around the outfall line that demonstrated the soil is not impacted (see Section 2).

3.2.9 Water Hazard Protection

26 Caution signs shall be installed along the top of the riverbank that state the following: "CAUTION -
27 DROWNING HAZARD, LIFE VEST REQUIRED WITHIN 5 FEET OF WATER". U.S. Coast Guard
28 approved life jackets or buoyant work vests will be issued to personnel performing working at the
29 riverbank and river. This includes work performed on the east riverbank to remove abandoned outfall line
30 debris from the middle of the river. In addition, ring buoys with at least 90 feet of line will be staged on
31 posts near the top of the riverbank. Distances between buoys will not exceed 200 feet. A lifesaving skiff
32 or boat will be staged within the work area where it is immediately available to the river.

3.3 REMEDIAL EXCAVATION (PART TWO)

Remedial excavation activities associated with this work include clearing and grubbing, removal of contaminated concrete, access ramp excavation, dumped rock fill access drive construction, overburden removal and stockpiling, cutting of the existing sheet piling (as required), removal of abandoned outfall line, excavation of potentially impacted soil from beneath the abandoned outfall line, hauling of pipe, and impacted soil on-site, SP7 stockpile area, removal of lead from pipe joints on-site outside the SP-7 stockpile area, usage of the haul routes, excavation dewatering, real-time monitoring and certification sampling, backfill of excavated area, radio and cell phone communications, dust control and restoring the work area. Remedial excavation activities are scheduled to start in early fall of 2004.

3.3.1 Clearing and Grubbing

Clearing of trees and other vegetation along the bank of the Great Miami River within the excavation area will be required for the Part Two remediation of the abandoned outfall line. Due to difficulty in accessing the lower parts of the riverbank, the clearing of some of the large trees may not take place until the ramp providing access to the outfall structure has been excavated. Clearing may be required if work is performed in any area where the off-site landowner has not harvested his crops. No grubbing will be required for Part Two remediation of the abandoned outfall line.

3.3.2 Removal of Contaminated Concrete

Riprap and broken concrete have been placed to armor the riverbank in the vicinity of the outfall structure from the erosive forces of the river. Since some of the broken concrete is radioactively contaminated, the riprap and broken concrete in the vicinity of the abandoned outfall structure will be removed and transported for proper disposal. Hereafter the riprap and contaminated broken concrete will collectively be called broken concrete.

Broken concrete will be removed within the limits shown on design drawing 99X-5500-G-00806 for proper disposition.

Initially, broken concrete will be removed from along the riverbank from the top of the riverbank to the top of the abandoned outfall structure (approximately at elevation 526 feet) to allow for the excavation of the access ramp. Broken concrete will be hauled to the site for proper disposition. Broken concrete failing WAO visual criteria for OSDF WAC will be segregated and placed in the SP-7 stockpile for subsequent off-site disposal. Otherwise, broken concrete will be hauled to the site for disposition in the OSDF. Silt fence will then be installed near the bottom of the sections of riverbank where the broken

1 concrete was removed. Real-time monitoring will be performed and soil samples will be collected from
2 the portion of the riverbank where broken concrete was removed. After the access ramp has been
3 excavated, silt fence will then be installed along the top of the riverbank.

4
5 The broken concrete will then be removed from the sides of the abandoned outfall line structure down
6 along the riverbank and out into the riverbed as far as the excavator can safety reach. Broken concrete
7 will be hauled to the site for proper disposition either in the OSDF or off-site based on WAO visual
8 criteria. New dumped rock will be placed from the bottom of the riverbed up the riverbank, to at least
9 elevation 528 feet, almost immediately after the broken concrete has been removed to minimize the
10 duration in which the riverbank is not protected (see Section 3.5 for restoration details). Prior to any new
11 dumped rock fill being placed to protect the riverbank, soil samples will be collected at locations along
12 the riverbank. Dumped rock fill will be placed along both sides of the abandoned outfall structure far
13 enough to establish a portion of the 15-foot wide dumped rock fill access drive so that the excavator can
14 reach additional broken concrete surrounding the abandoned outfall line structure. This process of
15 removing broken concrete that can safety be reached with the excavator then constructing more of the
16 dumped rock fill access drive so that additional broke concrete can be reached will be repeated until the
17 broken concrete surrounding the abandoned outfall structure is removed (see Section 3.3.4 for dumped
18 rock fill access drive).

19
20 Additional dumped rock will be placed up to the top of the riverbank at the end of Part Two remediation
21 and restoration, after pre-certification sample results show remaining soils are below OU5 off-site FRLs.

22 23 3.3.3 Access Ramp Excavation

24 A ramp will be excavated in the riverbank to provide access to the excavation of the abandoned outfall line
25 at the bottom of the riverbank and riverbed. The ramp will be excavated from top of riverbank elevation
26 $538 \pm$ feet to the riverbank at approximately elevation $526 \text{ feet} \pm$ (see drawing 99X-5500-G-00806). This
27 ramp will be cut at a 4 horizontal to 1 vertical slope. Soil directly under broken concrete will be scanned
28 prior to disposition. If soil fails precertification, then it will be excavated as impacted material and disposed
29 of at the OSDF unless WAO determines that it contains product or otherwise does not meet the
30 OSDF WAC. In that case, the soil will be hauled to SP-7 for subsequent off-site disposition.
31 The excavation of the ramp will not include pipe removal or excavation of soil adjacent to the abandoned
32 outfall line (see Section 3.3.5 for removal of the abandoned outfall line and associated soils).

Four to six inches of ODOT #617 or #304 limestone will be placed over the excavated ramp. No geotextile separator will be placed on the ramp since this would hinder the removal of the underlying pipeline.

In order to properly excavate the access ramp, portions of the sheet piling will be cut to 1-foot below the ramp profile elevation.

3.3.4 Dumped Rock Fill Access Drive

At the bottom of the access ramp, a dumped rock fill access drive will be constructed out into the river around the abandoned discharge structure extending into the river. This dumped rock fill access drive will be constructed using ODOT Type B dumped rock fill and be of sufficient width for excavation equipment to be safely staged on top of the dumped rock fill access drive (approximately 15 feet wide). The dumped rock fill access drive will be improved by choking stone into the top surface, if required.

The dumped rock fill access drive at the riverbank will be constructed out into the river in stages to allow the removal of additional broken concrete from the river bottom as it becomes reachable by the excavator. The dumped rock fill access drive will effectively provide a place to position the excavator to get the broken concrete located further out in the river without requiring that the new rock fill be removed later.

The majority of the dumped rock fill access drive will remain in-place at the end of the Part Two remediation of the abandoned outfall line and will remain in the river as additional protection to the active outfall line structure located immediately downstream of the abandoned outfall line structure.

3.3.5 Removal of Abandoned Outfall Line

The abandoned outfall line in the vicinity of the river and riverbank will generally be excavated from east to west from the river to just west of the riverbank (approximately at Station 37+56).

Prior to removing the piping, the broken concrete surrounding the abandoned outfall structure will be removed and the dumped rock fill access drive will be constructed (see Sections 3.3.2. and 3.3.4). Broken concrete will be hauled to the site for proper disposition. Broken concrete failing WAO visual criteria for OSDF WAC will be segregated and placed in the SP-7 stockpile for subsequent off-site disposal. Otherwise, broken concrete will be hauled to the site for disposition in the OSDF. To remove the abandoned outfall line east of the toe of the riverbank within the outfall structure, the structural steel cross-ties holding the sheeting piling on both sides of the pipeline together will be severed and removed.

1 Then the overburden crushed rock fill will be removed as impacted material and hauled to the site for
2 proper disposition either in the OSDF or off-site based on WAO visual criteria, since it is not known
3 exactly where the contaminated broken concrete was placed. Then the pipeline and crushed rock fill
4 adjacent to the pipe between the sheet pilings will be removed from the east end of the outfall structure to
5 approximately the base of the access ramp. Pipe will be removed in whole sections if possible. The pipe
6 will be removed and hauled to SP-7 for subsequent off-site disposition. Lead from the pipe joints will be
7 removed from the pipes and segregated for disposition to an off-site location.

8
9 Prior to continuing the excavation westward to remove additional pipe, the open trench between the sheet
10 piling that extends out into the river will be backfilled with Type B dumped rock fill. The trench will
11 then be extended westward to remove additional pipe sections and bedding material to approximately
12 Station 37+56. Soil removed above the abandoned outfall line will be stockpiled within the work area for
13 as backfill material. Soil adjacent to and underneath the abandoned outfall line will be hauled to SP-7 for
14 subsequent off-site disposition.

15
16 Due to the physical properties of the soil, the excavation will be laid back on both sides of the excavation
17 to a slope no steeper than 1 horizontal to 1 vertical slope. This slope is intended to provide access for
18 construction equipment only. If personnel are required to enter the trench excavation, side slope
19 configuration shall be determined by an OSHA competent person in accordance with OSHA Trenching
20 and Excavation Standards Subpart P.

21
22 Soil sampling for certification and Real Time measurements will be performed to verify that the impacted
23 material has been removed prior to backfill placement (see Section 3.4).

24
25 The subsequent trench excavation will be backfilled with soil in 8 inch \pm 1-inch loose lifts to establish the
26 riverbank. Backfill will be compacted to 90% of standard proctor maximum density and within moisture
27 content \pm 3 percentage points of optimum moisture content as determined by ASTM D698
28 (see Section 3.3.8 for backfill).

29
30 Debris from the abandoned outfall line is also located out in the middle of the river (visible when the
31 water level is low). It consists of a concrete slab and at least one section of pipe that will be removed and
32 hauled to SP-7 for subsequent off-site disposition. Due to the main channel of the river being between
33 this debris and the work area at the outfall structure, access to remove this debris could be better obtained
34 from the east bank of the river. The property owner on the east side of the Great Miami River is

Colerain Township. Efforts are ongoing to obtain permission to access the river through the Colerain Township property to remove the abandoned outfall line debris in the middle of the river. This will require cutting a temporary access ramp to the river on their property. This access will only be required during the limited time required to stage construction equipment and remove the debris. Thereafter, the riverbank will be restored approximately to its original condition. Access to the Colerain Township property will be provided through a Southern Ohio Water Company private drive. The material will be hauled to the site using the Southwestern Ohio Water Company's private drive then public roads as shown on Figure 3-1.

3.3.6 Haul Route

The haul route from the outfall structure work area will traverse from the excavation site along the improved off-site haul road south of Stricker's Grove (see Drawings 99X-5500-G-00807 and 99X-5500-G-00799), across State Route 128, along the off-site haul road improved during Part One site preparation, along the relocated North Entrance Road, along the OSDF Construction Lay-Down Area Haul Road north of the OSDF, through the OMTA Access Control Area, to the SP-7 stockpile area. From the SP-7 stockpile area, contaminated concrete, pipe, bedding material and soil will be loaded for disposition at an off-site location. See drawing 99X-5500-G-00802 for the State Route 128 Crossing Plan that includes signage and apron upgrade details.

Figure 3-1 shows the route for hauling the abandoned outfall debris located in the middle of the river from the river back to the FCP. No road improvements are required to establish this haul route.

3.3.7 Excavation Dewatering

For Part Two remediation of the abandoned outfall line, there are three primary activities that will disturb soil and require construction water controls. The first activity is the removal of broken concrete along the riverbank in the vicinity of the outfall structure. The second activity is the construction of a ramp that will provide access to the general work area along the riverbank and discharge structure that protrudes into the main channel. The third activity is the removal of the pipeline in the vicinity of the river and riverbank.

Silt fence will be installed near the bottom of the sections of riverbank where the broken concrete was removed prior to excavating the access ramp.

1 Since the discharge end of the pipeline is sometimes submerged, the ability to control construction water
2 in the vicinity of the river and the riverbank during the removal of the pipeline will largely dependent on
3 the water level of the Great Miami River. Part Two remediation of the abandoned outfall line is planned
4 for early fall when the river level is historically at the lowest. However, this is no certainty that the river
5 level will be low at the actual time of remediation.

6
7 If the river level is low enough, silt fence will be installed between the discharge end of the pipeline and
8 the river. If the river level is above the top of the abandoned discharge structure (approximately 526 feet
9 elevation) then excavation of the pipe in the river will be postponed until the river level has dropped to
10 below the top of the discharge structure. If the river level is low enough to facilitate work yet high
11 enough to cover part or all of the outfall line, then a silt fence will not be installed around the end of the
12 pipeline and the excavation in the river will proceed unabated. In order to minimize the erosion, the work
13 will be performed in a manner that will minimize the disturbance of materials below water. This includes
14 removing pipe sections in whole (if practical) and making the fewest number of cuts with the excavator
15 bucket as necessary to get material around the pipeline.

16
17 For work along the riverbank and at the discharge structure, runoff from the excavation will be filtered
18 through a silt fence, if possible, and discharged into the Great Miami River.

19
20 Construction water may be collected in the trench excavation west of the riverbank once the section of
21 abandoned outfall line extending into the river has been removed and backfill material placed within the
22 subsequent trench. If water within the trench west of the riverbank is not being recharged too rapidly by
23 either groundwater or river water, then this water will be tested and disposed accordingly, as follows.

24 Water will be sampled from within the excavation and tested. The points of disposition for water
25 collected within the excavation not requiring treatment will be the Great Miami River via the active
26 24-inch HDPE outfall line. The point of disposition for water collected within the excavation that
27 requires treatment will be the FCP's Wastewater Treatment System. The testing standard to be used to
28 determine the selection of disposition will be total uranium. If the excavation water does not exceed the
29 NPDES total suspended solids requirement and 30 ppb total uranium, then the water will be pumped to
30 the Great Miami River via the 24-inch HDPE outfall line as accessed through manhole SMH 183. If the
31 total uranium concentration in the excavation water exceeds 30 ppb, then the water will be pumped and
32 trucked back to the FCP for treatment. Volume of water discharged to the river via manhole SMH 183 or
33 delivered to the site for treatment will be recorded.

To control sediment in the construction water, a sump will be excavated at the down-gradient end of the open trench before the end of the day's excavation activities. If the water discharged from the sump fails to meet NPDES requirements for total suspended solids, other methods to reduce the total suspended solids concentration may be employed. The same portable gasoline powered pump and flexible hose that was procured for Part One remediation will be available for Part Two remediation if water is pumped to manhole SMH183.

If water within the excavation recharges at too great a rate, then the trench will not be dewatered; rather the excavation and backfill will be performed with standing water at the bottom of the trench. This may require modification to the side slopes or to the material used for filling the trench once the pipe and associated impacted soils have been removed.

3.3.8 Backfill

Once the excavation is complete and preliminary analysis and real-time monitoring has confirmed the absence of contaminated soil in an excavated section of trench, that section of trench will be backfilled using the stockpiled overburden soil or material borrowed from the off-site source. The backfill material will be compacted in-place to a density as noted on drawing 99X-5500-G-00806. This backfill material will be placed up to 6-inch below the surrounding existing grade (the grade after the topsoil was stripped from the surface). Since material was removed from the excavation, there may be a deficit of backfill material. Sand may be used as supplemental backfill material and may be obtained from obtained from nearby sand and gravel pits

Welch Sand and Gravel Pit Ross Plant located on the westward wide of State Route 128. Topsoil will be procured from an off-site source with input from the landowner.

3.3.9 Communications

A site radio or cell phone will be available within the on and off-site work areas to provide communications with the site for emergencies, severe weather notices and heat-stress stay time information.

3.4 PRECERTIFICATION AND CERTIFICATION SAMPLING

After removal of the pipeline, bedding material and over excavation of soil, in accordance with the typical trenching and backfilling detail shown on design drawing 99X-5500-G-00778, the excavator will periodically retrieve a bucket load of soil from the bottom of the excavation at every 25-feet of linear

trench, so that real-time monitoring can be performed. Soil samples for certification will be taken on the remaining soil as described in the Certification Design Letter (CDL). This way the monitoring and sampling can be performed at the surface so that workers will not have to enter the trench excavation. HPGe tripod measurements will be utilized to perform the real-time monitoring in lieu of the excavator mounted system (EMS) due to the lack of availability of the EMS, which is being fully utilized in the AWAC areas of the production area. Each bucket-load of material will be placed next to the trench to form a circular pad no less than 6 feet in diameter and 6 inches in thickness for HPGe measurements at a 15-cm detector height. The HPGe tripod measurement over the formed circular pad will determine if this soil is less than three times FRLs for uranium, thorium-232, and radium-226. If these results indicate less than 3 times FRLs for uranium, thorium-232, and radium-226, then the trench will be backfilled with the overburden material described above. If these results indicate more than 3 times FRLs for uranium, thorium-232, and radium-226, then the circular pad and underlying soil will be excavated for disposal and additional material will be excavated from the bottom of the trench until HPGE measurements confirm soil to contain uranium, thorium-232 and radium-226 at less than three times FRLs.

Certification samples will also be collected for chemical and radiological analysis from the excavator bucket that has retrieved soil from the bottom of the excavation as described above. The location of each sample point will be surveyed and recorded as documented in the associated CDL. In addition to the pre-defined locations, during excavation of the pipe and bedding material, a flag will be posted along the northern construction fence line at the easting corresponding to any stained soil, and/or broken, cracked, or disjointed piping. As discussed in the Certification Design Letter for Area 9 Phase III - Part One, biased samples will be collected at each of these flagged locations from the floor, and both the north and south sidewall approximately one foot from the floor of the excavation. The complete certification approach will be described in detail in the CDL and associated Certification Sampling Project Specific Plan that will be submitted along with this plan.

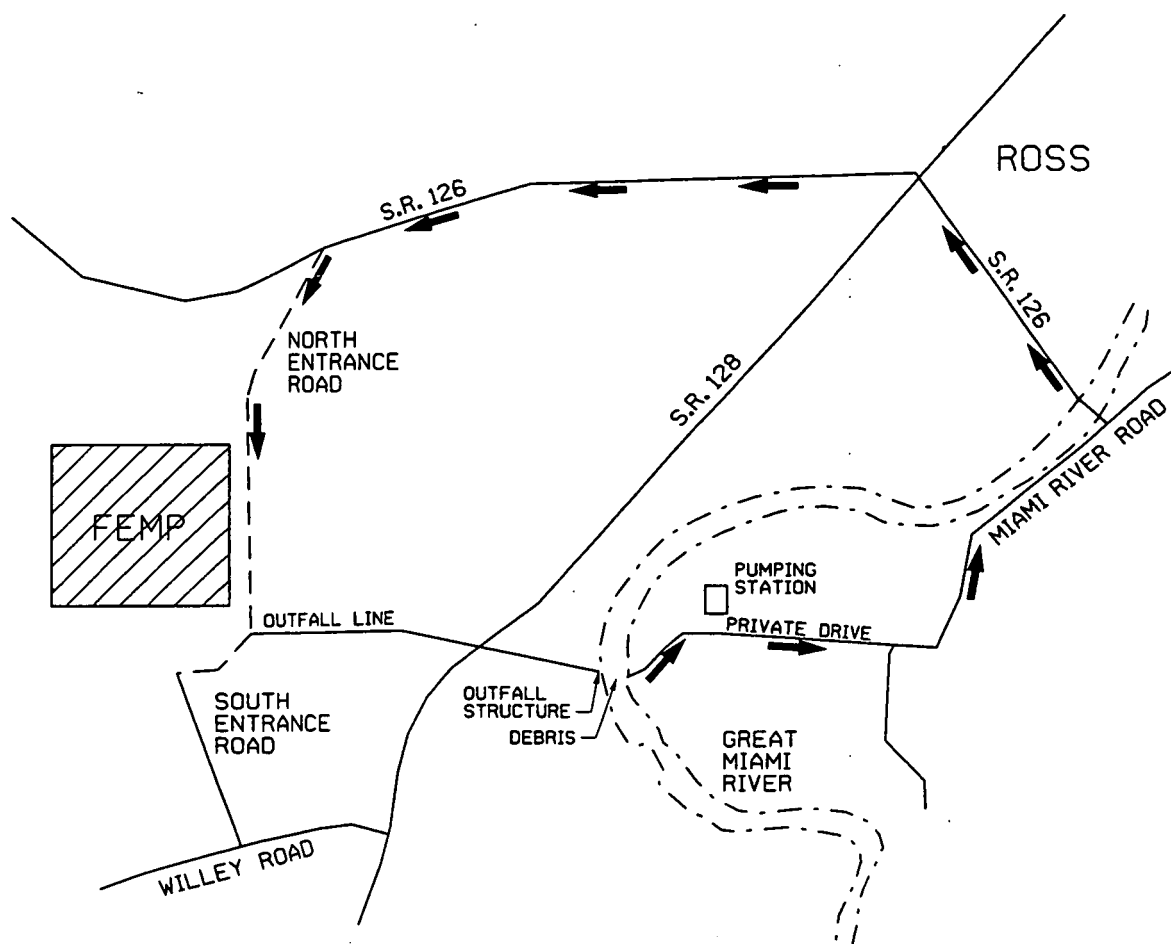
Once the real-time monitoring has confirmed the soil to contain uranium, thorium-232 and radium-226 at less than three times FRLs that section of trench will be backfilled with the stockpiled overburden material or as supplemented from off-site borrow areas (see Section 3.3.8). Within the entire project boundary, soil samples will be collected at the surface for certification as described in the CDL.

3.5 RESTORATION

Restoration will consist of reestablishing the work area to a similar condition the land was in prior to the remediation. Any restoration performed in the 100-year Floodplain of the Great Miami River must not

raise the surface elevation above the pre-work elevations. Restoration activities are scheduled to start in October 2004. Seeding of disturbed areas outside of cropped areas may be performed in October/November 2004. Cropped areas will not be seeded per the landowner's request. The off-site haul route will be left active for hauling and access activities associated with Part Three remediation of the abandoned outfall line.

ODOT Type B dumped rock fill will be placed along the riverbank from the riverbed upward to the top of the slope. Sufficient dumped rock will be placed at the toe of the slope to effectively anchor the dumped rock placed on the riverbank. AASHTO No. 2 Stone may be placed on portions of the riverbank to stabilize fine-grained soils prior to placing the dumped rock fill, if needed. Part of the dumped rock fill access drive, comprised mostly of ODOT Type B dumped rock fill, may be removed at the end of restoration and used as supplemental material for protecting the riverbank. Since the contaminated broken concrete along the riverbank and outfall structure will be removed, none of the broken concrete will be used as supplemental material to protect the riverbank.



GREAT MIAMI RIVER
EAST RIVERBANK HAUL ROUTE

FIGURE 3-1

REFERENCES

- U.S. Department of Energy, 1992, "Removal of Contaminated Rubble Near the FEMP Wastewater Discharge in the Great Miami River", Final, Fernald Environmental Management Project, DOE, Fernald Area Office, Cincinnati, Ohio.
- U.S. Department of Energy, 1993, "Project Specific Plan for the Great Miami Riverbank Characterization," Final, Fernald Environmental Management Project, DOE, Fernald Area Office, Cincinnati, Ohio.
- IT Corporation, 1993, "Radiation Survey Great Miami River Bank," Fernald Closure Project, DOE, Fernald Area Office, Cincinnati, Ohio.
- U.S. Department of Energy, 1993, "Supplement to Project Specific Plan for the Great Miami Riverbank Characterization," Final, Fernald Environmental Management Project, DOE, Fernald Area Office, Cincinnati, Ohio.
- U.S. Department of Energy, 1996, "Record of Decision for Remedial Actions at Operable Unit 5," Final, Fernald Environmental Management Project, DOE, Fernald Area Office, Cincinnati, Ohio.
- U.S. Department of Energy, 2002, "Project Specific Plan for Predesign Investigation of Area 5," Revision 0, Fernald Environmental Management Project, DOE, Fernald Area Office, Cincinnati, Ohio.
- U.S. Department of Energy, 2003, "Miscellaneous Small Structures Phase II Implementation Plan for Above-Grade Decontamination and Dismantlement," Revision 0 PCN 1, Fernald Closure Project, DOE, Fernald Area Office, Cincinnati, Ohio.
- U.S. Department of Energy, 2004, "Project Specific Plan for Area 9, Phase III Outfall Ditch Predesign Investigation," Revision 0, Fernald Environmental Management Project, DOE, Fernald Area Office, Cincinnati, Ohio.

APPENDIX A
LIST OF DRAWINGS AND TECHNICAL SPECIFICATIONS

APPENDIX A

LIST OF DRAWINGS AND TECHNICAL SPECIFICATIONS

Design Drawings

Drawing 99X-5500-G-00775	Remediation of Abandoned Outfall Line Master Layout Plan
Drawing 99X-5500-G-00797	Remediation of Abandoned Outfall Line, Haul Road Plan and Details
Drawing 99X-5500-G-00778	Remediation of Abandoned Outfall Line Details
Drawing 99X-5500-G-00807	Part Two Haul Road (Sheet 1 of 2)
Drawing 99X-5500-G-00799	Part Two Haul Road (Sheet 2 of 2)
Drawing 99X-5500-G-00806	Part Two Outfall Excavation Plan
Drawing 99X-5500-G-00808	Part Two Outfall Restoration Plan
Drawing 99X-5500-G-00809	Part Two Details
Drawing 99X-5500-G-00802	ST. RT. 128 Crossing Traffic Plan

Technical Specifications

Excavation for Remediation Technical Specifications

01215	Traffic Control
02205	Impacted Material Excavation
02206	Earthwork for Remediation
02275	Surface Water Management and Erosion Control for Remediation

OSDF Technical Specifications

02100	Surveying
02200	Earthwork
02230	Road Construction
02270	Surface Water Management and Erosion Control
02714	Geotextile
02930	Vegetation

APPENDIX B

HISTORICAL AND CHARACTERIZATION DATA

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APPENDIX B
TABLE B-1
HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11168	125191	0	1.5	Thorium, Total	18	UNV	mg/kg
11168	125191	0	1.5	Uranium, Total	11	UNV	mg/kg
11168	125192	1.5	3	Thorium, Total	18	UNV	mg/kg
11168	125192	1.5	3	Uranium, Total	11	UNV	mg/kg
11168	125193	3	4.5	Thorium, Total	18	UNV	mg/kg
11168	125193	3	4.5	Uranium, Total	11	UNV	mg/kg
11168	125194	4.5	6	Thorium, Total	18	UNV	mg/kg
11168	125194	4.5	6	Thorium, Total	18	UNV	mg/kg
11168	125195	4.5	6	Uranium, Total	11	UNV	mg/kg
11168	125195	4.5	6	Uranium, Total	11	UNV	mg/kg
11168	125196	6	7.5	Thorium, Total	18	UNV	mg/kg
11168	125196	6	7.5	Uranium, Total	11	UNV	mg/kg
11168	125197	7.5	9	Thorium, Total	18	UNV	mg/kg
11168	125197	7.5	9	Uranium, Total	11	UNV	mg/kg
11168	125198	9	10.5	Thorium, Total	18	UNV	mg/kg
11168	125198	9	10.5	Uranium, Total	11	UNV	mg/kg
11168	125199	10.5	12	Thorium, Total	18	UNV	mg/kg
11168	125199	10.5	12	Uranium, Total	11	UNV	mg/kg
11168	125201	12	13	Uranium, Total	0.90	J	mg/kg
11168	125201	12	13	Cesium-137	0.2	U	pCi/g
11168	125200	12	13	Gross Alpha	3.6	UNV	pCi/g
11168	125201	12	13	Gross Beta	7.1	NV	pCi/g
11168	125201	12	13	Radium-226	0.3	J	pCi/g
11168	125201	12	13	Radium-228	0.7	U	pCi/g
11168	125201	12	13	Strontium-90	0.5	UJ	pCi/g
11168	125201	12	13	Thorium-228	0.3	J	pCi/g
11168	125201	12	13	Thorium-230	0.6	J	pCi/g
11168	125201	12	13	Thorium-232	0.2	J	pCi/g
11168	125201	12	13	Uranium-234	0.2	J	pCi/g
11168	125202	12	13	Uranium-235/236	0.1	UJ	pCi/g
11168	125202	12	13	Uranium-238	0.3	J	pCi/g
11168	125201	12	13	Gross Alpha	17	UNV	pCi/g
11168	125201	12	13	Gross Beta	25	NV	pCi/g
11168	125201	12	13	Thorium, Total	18	UNV	mg/kg
11168	125201	12	13	Technetium-99	1	UJ	pCi/g
11168	125204			Gross Alpha	0.28	UNV	pCi/g
11168	125204			Gross Beta	0.6	UNV	pCi/g
11169	125114	0	1.5	Thorium, Total	18	UNV	mg/kg
11169	125114	0	1.5	Uranium, Total	11	UNV	mg/kg

APPENDIX B

TABLE B-1

HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11169	125115	1.5	3	Thorium, Total	18	UNV	mg/kg
11169	125115	1.5	3	Uranium, Total	11	UNV	mg/kg
11169	125116	3	4.5	Thorium, Total	18	UNV	mg/kg
11169	125116	3	4.5	Uranium, Total	11	UNV	mg/kg
11169	125117	4.5	6	Thorium, Total	18	UNV	mg/kg
11169	125117	4.5	6	Uranium, Total	11	UNV	mg/kg
11169	125118	6	7.5	Thorium, Total	18	UNV	mg/kg
11169	125118	6	7.5	Uranium, Total	11	UNV	mg/kg
11169	125123	7.5	9	Uranium, Total	0.90	J	mg/kg
11169	125123	7.5	9	Radium-226	0.25	UJ	pCi/g
11169	125119	7.5	9	Cesium-137	0.2	U	pCi/g
11169	125123	7.5	9	Gross Alpha	5.7	NV	pCi/g
11169	125123	7.5	9	Gross Alpha	3.3	NV	pCi/g
11169	125123	7.5	9	Gross Beta	9.9	NV	pCi/g
11169	125123	7.5	9	Radium-228	0.6	U	pCi/g
11169	125123	7.5	9	Strontium-90	0.5	UJ	pCi/g
11169	125123	7.5	9	Thorium-228	0.3	J	pCi/g
11169	125123	7.5	9	Thorium-230	0.8	J	pCi/g
11169	125123	7.5	9	Thorium-232	0.3	J	pCi/g
11169	125123	7.5	9	Uranium-234	0.3	J	pCi/g
11169	125120	7.5	9	Uranium-235/236	0.1	UJ	pCi/g
11169	125120	7.5	9	Uranium-238	0.3	J	pCi/g
11169	125123	7.5	9	Gross Beta	24	UNV	pCi/g
11169	125123	7.5	9	Thorium, Total	18	UNV	mg/kg
11169	125123	7.5	9	Technetium-99	1	U	pCi/g
11169	125121	9	10.5	Thorium, Total	18	UNV	mg/kg
11169	125121	9	10.5	Uranium, Total	11	UNV	mg/kg
11169	125122	10.5	12	Thorium, Total	18	UNV	mg/kg
11169	125122	10.5	12	Uranium, Total	11	UNV	mg/kg
11169	125124	12	13	Thorium, Total	18	UNV	mg/kg
11169	125124	12	13	Uranium, Total	11	UNV	mg/kg
11170	125031	0	1.5	Gross Beta	53.5	NV	pCi/g
11170	125031	0	1.5	Radium-226	0.25	UJ	pCi/g
11170	125031	0	1.5	Cesium-137	0.2	U	pCi/g
11170	125031	0	1.5	Gross Alpha	4.7	NV	pCi/g
11170	125031	0	1.5	Plutonium-238	0.2	UJ	pCi/g
11170	125031	0	1.5	Plutonium-239/240	0.2	UJ	pCi/g
11170	125031	0	1.5	Radium-228	0.5	U	pCi/g
11170	125031	0	1.5	Strontium-90	0.5	UJ	pCi/g

**APPENDIX B
TABLE B-1
HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO**

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11170	125031	0	1.5	Thorium-228	0.6	J	pCi/g
11170	125031	0	1.5	Thorium-230	0.9	J	pCi/g
11170	125031	0	1.5	Thorium-232	0.5	J	pCi/g
11170	125031	0	1.5	Technetium-99	1	UJ	pCi/g
11171	125000	0	1.5	Thorium, Total	18	UNV	mg/kg
11171	125000	0	1.5	Uranium, Total	11	UNV	mg/kg
11171	125001	1.5	3	Thorium, Total	18	UNV	mg/kg
11171	125001	1.5	3	Uranium, Total	11	UNV	mg/kg
11171	125002	4.5	6	Thorium, Total	18	UNV	mg/kg
11171	125002	4.5	6	Uranium, Total	11	UNV	mg/kg
11171	125003	6	7.5	Thorium, Total	18	UNV	mg/kg
11171	125003	6	7.5	Uranium, Total	11	UNV	mg/kg
11171	125004	7.5	9	Thorium, Total	18	UNV	mg/kg
11171	125004	7.5	9	Uranium, Total	11	UNV	mg/kg
11171	125005	9	10.5	Thorium, Total	18	UNV	mg/kg
11171	125005	9	10.5	Uranium, Total	11	UNV	mg/kg
11171	125006	10.5	12	Thorium, Total	18	UNV	mg/kg
11171	125006	10.5	12	Uranium, Total	11	UNV	mg/kg
11171	125008	12	13	Uranium, Total	2.70	J	mg/kg
11171	125008	12	13	Radium-226	0.25	UJ	pCi/g
11171	125008	12	13	Cesium-137	0.8	-	pCi/g
11171	125008	12	13	Gross Alpha	5.8	NV	pCi/g
11171	125008	12	13	Gross Beta	8.2	NV	pCi/g
11171	125008	12	13	Plutonium-238	0.2	UJ	pCi/g
11171	125008	12	13	Plutonium-239/240	0.2	UJ	pCi/g
11171	125008	12	13	Radium-228	0.6	U	pCi/g
11171	125008	12	13	Strontium-90	0.6	UJ	pCi/g
11171	125008	12	13	Thorium-228	0.2	J	pCi/g
11171	125008	12	13	Thorium-230	0.5	J	pCi/g
11171	125008	12	13	Thorium-232	0.2	J	pCi/g
11171	125008	12	13	Uranium-234	0.9	J	pCi/g
11171	125007	12	13	Uranium-235/236	0.1	UJ	pCi/g
11171	125008	12	13	Uranium-238	0.9	J	pCi/g
11171	125008	12	13	Thorium, Total	18	UNV	mg/kg
11171	125008	12	13	Technetium-99	1	UJ	pCi/g
11172	125165	0	1.5	Thorium, Total	18	NV	mg/kg
11172	125165	0	1.5	Uranium, Total	11	NV	mg/kg
11172	125166	1.5	3	Thorium, Total	18	NV	mg/kg
11172	125166	1.5	3	Thorium, Total	18	UNV	mg/kg

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TABLE B-1
HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11172	125167	1.5	3	Uranium, Total	11	NV	mg/kg
11172	125167	1.5	3	Uranium, Total	11	UNV	mg/kg
11172	125170	3	4.5	Thorium, Total	18	NV	mg/kg
11172	125170	3	4.5	Uranium, Total	11	NV	mg/kg
11172	125171	4.5	6	Thorium, Total	18	NV	mg/kg
11172	125171	4.5	6	Uranium, Total	11	NV	mg/kg
11172	125172	6	7.5	Thorium, Total	18	NV	mg/kg
11172	125172	6	7.5	Uranium, Total	11	NV	mg/kg
11172	125173	7.5	9	Thorium, Total	18	NV	mg/kg
11172	125173	7.5	9	Uranium, Total	11	NV	mg/kg
11172	125177	9	10.5	Uranium, Total	2.10	J	mg/kg
11172	125177	9	10.5	Gross Beta	13.2	NV	pCi/g
11172	125177	9	10.5	Radium-226	0.25	UJ	pCi/g
11172	125177	9	10.5	Cesium-137	0.2	U	pCi/g
11172	125177	9	10.5	Plutonium-238	0.2	UJ	pCi/g
11172	125177	9	10.5	Plutonium-239/240	0.2	UJ	pCi/g
11172	125177	9	10.5	Radium-228	0.6	U	pCi/g
11172	125177	9	10.5	Strontium-90	0.5	UJ	pCi/g
11172	125177	9	10.5	Thorium-228	0.7	J	pCi/g
11172	125177	9	10.5	Thorium-230	1.3	J	pCi/g
11172	125177	9	10.5	Thorium-232	0.6	J	pCi/g
11172	125177	9	10.5	Uranium-234	0.6	J	pCi/g
11172	125177	9	10.5	Uranium-235/236	0.1	UJ	pCi/g
11172	125174	9	10.5	Uranium-238	0.7	J	pCi/g
11172	125177	9	10.5	Thorium, Total	18	NV	mg/kg
11172	125177	9	10.5	Gross Alpha	6	NV	pCi/g
11172	125177	9	10.5	Technetium-99	1	UJ	pCi/g
11172	125175	10.5	12	Thorium, Total	18	NV	mg/kg
11172	125175	10.5	12	Uranium, Total	11	NV	mg/kg
11172	125176	12	13	Thorium, Total	18	NV	mg/kg
11172	125176	12	13	Uranium, Total	11	NV	mg/kg
11174	125179	0	1.5	Thorium, Total	18	UNV	mg/kg
11174	125179	0	1.5	Uranium, Total	11	UNV	mg/kg
11174	125180	1.5	3	Thorium, Total	18	UNV	mg/kg
11174	125180	1.5	3	Uranium, Total	11	UNV	mg/kg
11174	125181	3	4.5	Thorium, Total	18	UNV	mg/kg
11174	125181	3	4.5	Thorium, Total	18	UNV	mg/kg
11174	125183	3	4.5	Uranium, Total	11	UNV	mg/kg
11174	125183	3	4.5	Uranium, Total	11	UNV	mg/kg

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TABLE B-1

HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11174	125184	4.5	6	Thorium, Total	18	UNV	mg/kg
11174	125184	4.5	6	Uranium, Total	11	UNV	mg/kg
11174	125185	6	7.5	Thorium, Total	18	UNV	mg/kg
11174	125185	6	7.5	Uranium, Total	11	UNV	mg/kg
11174	125189	7.5	9	Thorium, Total	18	UNV	mg/kg
11174	125186	7.5	9	Uranium, Total	11	UNV	mg/kg
11174	125189	9	10.5	Radium-226	0.25	UJ	pCi/g
11174	125189	9	10.5	Cesium-137	0.2	U	pCi/g
11174	125189	9	10.5	Gross Alpha	3.6	UNV	pCi/g
11174	125189	9	10.5	Gross Beta	7.5	NV	pCi/g
11174	125189	9	10.5	Plutonium-238	0.2	UJ	pCi/g
11174	125189	9	10.5	Plutonium-239/240	0.2	UJ	pCi/g
11174	125189	9	10.5	Radium-228	0.5	U	pCi/g
11174	125189	9	10.5	Strontium-90	0.6	UJ	pCi/g
11174	125187	9	10.5	Thorium-228	0.2	J	pCi/g
11174	125187	9	10.5	Thorium-230	0.5	J	pCi/g
11174	125188	9	10.5	Thorium-232	0.2	J	pCi/g
11174	125188	9	10.5	Uranium, Total	1.8	-	mg/kg
11174	125189	9	10.5	Gross Alpha	18	UNV	pCi/g
11174	125189	9	10.5	Gross Beta	26	NV	pCi/g
11174	125189	9	10.5	Thorium, Total	18	UNV	mg/kg
11174	125189	9	10.5	Technetium-99	1	UJ	pCi/g
11174	125182	10.5	12	Thorium, Total	18	UNV	mg/kg
11174	125182	10.5	12	Uranium, Total	11	UNV	mg/kg
11174	125190	12	13	Thorium, Total	18	UNV	mg/kg
11174	125190	12	13	Uranium, Total	11	UNV	mg/kg
11175	125088	0	1.5	Thorium, Total	18	UNV	mg/kg
11175	125088	0	1.5	Uranium, Total	11	UNV	mg/kg
11175	125089	1.5	3	Thorium, Total	18	UNV	mg/kg
11175	125089	1.5	3	Uranium, Total	11	UNV	mg/kg
11175	125090	3	4.5	Thorium, Total	18	UNV	mg/kg
11175	125090	3	4.5	Uranium, Total	11	UNV	mg/kg
11175	125091	4.5	6	Thorium, Total	18	UNV	mg/kg
11175	125091	4.5	6	Uranium, Total	11	UNV	mg/kg
11175	125092	6	7.5	Thorium, Total	18	UNV	mg/kg
11175	125092	6	7.5	Uranium, Total	11	UNV	mg/kg
11175	125093	7.5	9	Thorium, Total	18	UNV	mg/kg
11175	125093	7.5	9	Uranium, Total	11	UNV	mg/kg
11175	125112	9	10.5	Radium-226	0.25	UJ	pCi/g

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TABLE B-1
HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11175	125112	9	10.5	Cesium-137	0.2	U	pCi/g
11175	125094	9	10.5	Gross Alpha	3.6	UNV	pCi/g
11175	125112	9	10.5	Gross Beta	9.1	NV	pCi/g
11175	125112	9	10.5	Radium-228	0.6	-	pCi/g
11175	125112	9	10.5	Strontium-90	0.5	UJ	pCi/g
11175	125112	9	10.5	Thorium-228	0.3	J	pCi/g
11175	125112	9	10.5	Thorium-230	0.5	J	pCi/g
11175	125112	9	10.5	Thorium-232	0.3	J	pCi/g
11175	125112	9	10.5	Uranium, Total	1.7	-	mg/kg
11175	125112	9	10.5	Thorium, Total	18	UNV	mg/kg
11175	125112	9	10.5	Technetium-99	1	U	pCi/g
11175	125095	10.5	12	Thorium, Total	18	UNV	mg/kg
11175	125095	10.5	12	Thorium, Total	18	UNV	mg/kg
11175	125097	10.5	12	Uranium, Total	11	UNV	mg/kg
11175	125097	10.5	12	Uranium, Total	11	UNV	mg/kg
11175	125096	12	13	Thorium, Total	18	UNV	mg/kg
11175	125096	12	13	Uranium, Total	11	UNV	mg/kg
11176	125054	0	1.5	Gross Beta	18.6	NV	pCi/g
11176	125054	0	1.5	Radium-226	0.25	U	pCi/g
11176	125054	0	1.5	Cesium-137	0.2	U	pCi/g
11176	125054	0	1.5	Gross Alpha	5.9	NV	pCi/g
11176	125043	0	1.5	Radium-228	0.6	U	pCi/g
11176	125054	0	1.5	Strontium-90	1.6	UJ	pCi/g
11176	125054	0	1.5	Uranium, Total	2.2	-	mg/kg
11176	125054	0	1.5	Thorium, Total	18	UNV	mg/kg
11176	125054	0	1.5	Technetium-99	1	UJ	pCi/g
11176	125044	1.5	3	Thorium, Total	18	UNV	mg/kg
11176	125044	1.5	3	Uranium, Total	11	UNV	mg/kg
11176	125045	3	4.5	Thorium, Total	18	UNV	mg/kg
11176	125045	3	4.5	Uranium, Total	11	UNV	mg/kg
11176	125046	4.5	6	Thorium, Total	18	UNV	mg/kg
11176	125046	4.5	6	Uranium, Total	11	UNV	mg/kg
11176	125049	6	7.5	Thorium, Total	18	UNV	mg/kg
11176	125047	6	7.5	Thorium, Total	18	UNV	mg/kg
11176	125047	6	7.5	Uranium, Total	11	UNV	mg/kg
11176	125050	7.5	9	Thorium, Total	18	UNV	mg/kg
11176	125050	7.5	9	Uranium, Total	11	UNV	mg/kg
11176	125051	9	10.5	Thorium, Total	18	UNV	mg/kg
11176	125051	9	10.5	Uranium, Total	11	UNV	mg/kg

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TABLE B-1
HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in:feet)	Bottom Depth (in:feet)	Parameter	Result	Qualifier	Units
11176	125052	10.5	12	Thorium, Total	18	UNV	mg/kg
11176	125052	10.5	12	Uranium, Total	11	UNV	mg/kg
11176	125053	12	13	Thorium, Total	18	UNV	mg/kg
11176	125053	12	13	Uranium, Total	11	UNV	mg/kg
11177	125019	0	1.5	Gross Beta	15.7	NV	pCi/g
11177	125019	0	1.5	Cesium-137	0.2	U	pCi/g
11177	125019	0	1.5	Gross Alpha	8.1	NV	pCi/g
11177	125019	0	1.5	Plutonium-238	0.2	UJ	pCi/g
11177	125009	0	1.5	Plutonium-239/240	0.2	UJ	pCi/g
11177	125019	0	1.5	Radium-226	0.6	J	pCi/g
11177	125019	0	1.5	Radium-228	0.5	U	pCi/g
11177	125020	0	1.5	Strontium-90	0.5	UJ	pCi/g
11177	125019	0	1.5	Thorium-228	0.3	J	pCi/g
11177	125019	0	1.5	Thorium-230	0.3	J	pCi/g
11177	125019	0	1.5	Thorium-232	0.3	J	pCi/g
11177	125019	0	1.5	Uranium, Total	2.5	-	mg/kg
11177	125019	0	1.5	Thorium, Total	18	UNV	mg/kg
11177	125019	0	1.5	Thorium, Total	18	UNV	mg/kg
11177	125019	0	1.5	Technetium-99	1	UJ	pCi/g
11177	125010	1.5	3	Thorium, Total	18	UNV	mg/kg
11177	125010	1.5	3	Thorium, Total	18	UNV	mg/kg
11177	125011	1.5	3	Thorium, Total	18	UNV	mg/kg
11177	125021	1.5	3	Uranium, Total	11	UNV	mg/kg
11177	125012	3	4.5	Thorium, Total	18	UNV	mg/kg
11177	125012	3	4.5	Thorium, Total	18	UNV	mg/kg
11177	125022	3	4.5	Thorium, Total	18	UNV	mg/kg
11177	125023	3	4.5	Uranium, Total	11	UNV	mg/kg
11177	125023	3	4.5	Uranium, Total	11	UNV	mg/kg
11177	125013	4.5	6	Thorium, Total	18	UNV	mg/kg
11177	125013	4.5	6	Thorium, Total	18	UNV	mg/kg
11177	125025	4.5	6	Uranium, Total	11	UNV	mg/kg
11177	125014	6	7.5	Thorium, Total	18	UNV	mg/kg
11177	125014	6	7.5	Thorium, Total	18	UNV	mg/kg
11177	125026	6	7.5	Uranium, Total	11	UNV	mg/kg
11177	125015	7.5	9	Thorium, Total	18	UNV	mg/kg
11177	125015	7.5	9	Thorium, Total	18	UNV	mg/kg
11177	125027	7.5	9	Uranium, Total	11	UNV	mg/kg
11177	125016	9	10.5	Thorium, Total	18	UNV	mg/kg
11177	125016	9	10.5	Thorium, Total	18	UNV	mg/kg

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HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11177	125028	9	10.5	Uranium, Total	11	UNV	mg/kg
11177	125017	10.5	12	Thorium, Total	18	UNV	mg/kg
11177	125017	10.5	12	Thorium, Total	18	UNV	mg/kg
11177	125029	10.5	12	Uranium, Total	11	UNV	mg/kg
11177	125018	12	13	Thorium, Total	18	UNV	mg/kg
11177	125018	12	13	Thorium, Total	18	UNV	mg/kg
11177	125030	12	13	Uranium, Total	11	UNV	mg/kg
11178	125163	0	1.5	Uranium, Total	1.80	J	mg/kg
11178	125163	0	1.5	Gross Beta	22.1	NV	pCi/g
11178	125163	0	1.5	Radium-226	0.25	U	pCi/g
11178	125160	0	1.5	Cesium-137	0.3	-	pCi/g
11178	125163	0	1.5	Gross Alpha	8.1	NV	pCi/g
11178	125163	0	1.5	Radium-228	0.6	U	pCi/g
11178	125163	0	1.5	Strontium-90	0.5	UJ	pCi/g
11178	125163	0	1.5	Thorium-228	0.6	J	pCi/g
11178	125163	0	1.5	Thorium-230	0.9	J	pCi/g
11178	125163	0	1.5	Thorium-232	0.5	J	pCi/g
11178	125163	0	1.5	Uranium-234	0.7	J	pCi/g
11178	125163	0	1.5	Uranium-235/236	0.1	UJ	pCi/g
11178	125163	0	1.5	Uranium-238	0.6	J	pCi/g
11178	125163	0	1.5	Thorium, Total	18	NV	mg/kg
11178	125163	0	1.5	Technetium-99	1	U	pCi/g
11178	125161	1.5	3	Uranium, Total	53.8	NV	mg/kg
11178	125161	1.5	3	Thorium, Total	18	NV	mg/kg
11178	125162	3	4.5	Uranium, Total	24.1	NV	mg/kg
11178	125162	3	4.5	Thorium, Total	18	UNV	mg/kg
11179	125125	0	1.5	Thorium, Total	18	UNV	mg/kg
11179	125125	0	1.5	Thorium, Total	18	UNV	mg/kg
11179	125126	0	1.5	Uranium, Total	11	UNV	mg/kg
11179	125126	0	1.5	Uranium, Total	11	UNV	mg/kg
11179	125127	1.5	3	Thorium, Total	18	UNV	mg/kg
11179	125127	1.5	3	Uranium, Total	11	UNV	mg/kg
11179	125128	3	4.5	Thorium, Total	18	UNV	mg/kg
11179	125128	3	4.5	Uranium, Total	11	UNV	mg/kg
11179	125129	4.5	6	Thorium, Total	18	UNV	mg/kg
11179	125129	4.5	6	Uranium, Total	11	UNV	mg/kg
11179	125130	6	7.5	Thorium, Total	18	UNV	mg/kg
11179	125130	6	7.5	Uranium, Total	11	UNV	mg/kg
11179	125135	7.5	9	Uranium, Total	1.20	J	mg/kg

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TABLE B-1

HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11179	125135	7.5	9	Gross Beta	44.9	NV	pCi/g
11179	125135	7.5	9	Radium-226	0.25	UJ	pCi/g
11179	125135	7.5	9	Cesium-137	0.2	U	pCi/g
11179	125135	7.5	9	Gross Alpha	4.6	UNV	pCi/g
11179	125135	7.5	9	Radium-228	0.6	U	pCi/g
11179	125135	7.5	9	Strontium-90	0.5	UJ	pCi/g
11179	125135	7.5	9	Thorium-228	0.5	J	pCi/g
11179	125135	7.5	9	Thorium-232	0.5	J	pCi/g
11179	125135	7.5	9	Uranium-234	0.3	J	pCi/g
11179	125131	7.5	9	Uranium-235/236	0.1	UJ	pCi/g
11179	125135	7.5	9	Uranium-238	0.4	J	pCi/g
11179	125135	7.5	9	Thorium, Total	18	UNV	mg/kg
11179	125135	7.5	9	Technetium-99	1	U	pCi/g
11179	125135	7.5	9	Thorium-230	1	J	pCi/g
11179	125132	9	10.5	Thorium, Total	18	UNV	mg/kg
11179	125132	9	10.5	Uranium, Total	11	UNV	mg/kg
11179	125133	10.5	12	Thorium, Total	18	UNV	mg/kg
11179	125133	10.5	12	Uranium, Total	11	UNV	mg/kg
11179	125134	12	13	Thorium, Total	18	UNV	mg/kg
11179	125134	12	13	Uranium, Total	11	UNV	mg/kg
11180	125137	0	1.5	Thorium, Total	18	UNV	mg/kg
11180	125137	0	1.5	Uranium, Total	11	UNV	mg/kg
11180	125138	1.5	3	Thorium, Total	18	UNV	mg/kg
11180	125138	1.5	3	Thorium, Total	18	UNV	mg/kg
11180	125139	1.5	3	Uranium, Total	11	UNV	mg/kg
11180	125147	3	4.5	Uranium, Total	1.50	J	mg/kg
11180	125147	3	4.5	Gross Beta	17.1	NV	pCi/g
11180	125147	3	4.5	Cesium-137	0.2	U	pCi/g
11180	125147	3	4.5	Gross Alpha	6.9	NV	pCi/g
11180	125147	3	4.5	Plutonium-238	0.2	UJ	pCi/g
11180	125147	3	4.5	Plutonium-239/240	0.2	UJ	pCi/g
11180	125147	3	4.5	Radium-226	0.3	J	pCi/g
11180	125147	3	4.5	Radium-228	0.6	U	pCi/g
11180	125140	3	4.5	Strontium-90	0.7	UJ	pCi/g
11180	125147	3	4.5	Thorium-228	0.3	J	pCi/g
11180	125147	3	4.5	Thorium-230	0.4	J	pCi/g
11180	125147	3	4.5	Thorium-232	0.4	J	pCi/g
11180	125147	3	4.5	Uranium-234	0.4	J	pCi/g
11180	125147	3	4.5	Uranium-235/236	0.1	UJ	pCi/g

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TABLE B-1

HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11180	125147	3	4.5	Uranium-238	0.5	J	pCi/g
11180	125147	3	4.5	Thorium, Total	18	UNV	mg/kg
11180	125147	3	4.5	Technetium-99	1	UJ	pCi/g
11180	125141	4.5	6	Thorium, Total	18	UNV	mg/kg
11180	125141	4.5	6	Uranium, Total	11	UNV	mg/kg
11180	125142	6	7.5	Thorium, Total	18	UNV	mg/kg
11180	125142	6	7.5	Uranium, Total	11	UNV	mg/kg
11180	125143	7.5	9	Thorium, Total	18	UNV	mg/kg
11180	125143	7.5	9	Uranium, Total	11	UNV	mg/kg
11180	125144	9	10.5	Thorium, Total	18	UNV	mg/kg
11180	125144	9	10.5	Uranium, Total	11	UNV	mg/kg
11180	125145	10.5	12	Thorium, Total	18	UNV	mg/kg
11180	125145	10.5	12	Uranium, Total	11	UNV	mg/kg
11180	125146	12	13	Thorium, Total	18	NV	mg/kg
11180	125146	12	13	Uranium, Total	11	NV	mg/kg
11181	125159	0	1.5	Thorium, Total	18	UNV	mg/kg
11181	125151	0	1.5	Thorium, Total	18	UNV	mg/kg
11181	125151	0	1.5	Uranium, Total	53	NV	mg/kg
11181	125157	1.5	3	Uranium, Total	20.72	J	mg/kg
11181	125157	1.5	3	Gross Alpha	17.4	NV	pCi/g
11181	125157	1.5	3	Gross Beta	34.1	NV	pCi/g
11181	125157	1.5	3	Radium-226	0.25	UJ	pCi/g
11181	125157	1.5	3	Cesium-137	0.2	U	pCi/g
11181	125157	1.5	3	Radium-228	0.5	U	pCi/g
11181	125157	1.5	3	Strontium-90	0.5	UJ	pCi/g
11181	125157	1.5	3	Thorium-228	0.5	J	pCi/g
11181	125157	1.5	3	Thorium-230	0.8	J	pCi/g
11181	125157	1.5	3	Thorium-232	0.5	J	pCi/g
11181	125152	1.5	3	Uranium-234	6.9	J	pCi/g
11181	125157	1.5	3	Uranium-235/236	0.4	J	pCi/g
11181	125157	1.5	3	Uranium-238	6.9	J	pCi/g
11181	125157	1.5	3	Thorium, Total	18	UNV	mg/kg
11181	125157	1.5	3	Technetium-99	1	U	pCi/g
11181	125153	3	4.5	Thorium, Total	18	UNV	mg/kg
11181	125153	3	4.5	Uranium, Total	11	UNV	mg/kg
11181	125154	4.5	6	Thorium, Total	18	UNV	mg/kg
11181	125154	4.5	6	Uranium, Total	11	UNV	mg/kg
11181	125155	6	7.5	Thorium, Total	18	UNV	mg/kg
11181	125155	6	7.5	Uranium, Total	11	UNV	mg/kg

APPENDIX B

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TABLE B-1

HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11181	125156	7.5	9	Thorium, Total	18	UNV	mg/kg
11181	125156	7.5	9	Uranium, Total	11	UNV	mg/kg
11182	125074	0	1.5	Thorium, Total	18	UNV	mg/kg
11182	125074	0	1.5	Uranium, Total	11	UNV	mg/kg
11182	125075	1.5	3	Thorium, Total	18	UNV	mg/kg
11182	125075	1.5	3	Uranium, Total	11	UNV	mg/kg
11182	125076	3	4.5	Thorium, Total	18	UNV	mg/kg
11182	125076	3	4.5	Uranium, Total	11	UNV	mg/kg
11182	125086	4.5	6	Uranium, Total	2.10	J	mg/kg
11182	125077	4.5	6	Cesium-137	0.2	U	pCi/g
11182	125086	4.5	6	Gross Alpha	7.1	NV	pCi/g
11182	125086	4.5	6	Plutonium-238	0.2	UJ	pCi/g
11182	125086	4.5	6	Plutonium-239/240	0.2	UJ	pCi/g
11182	125086	4.5	6	Radium-226	0.3	J	pCi/g
11182	125086	4.5	6	Radium-228	0.6	-	pCi/g
11182	125086	4.5	6	Strontium-90	1.1	UJ	pCi/g
11182	125086	4.5	6	Thorium-228	0.4	J	pCi/g
11182	125086	4.5	6	Thorium-230	0.6	J	pCi/g
11182	125086	4.5	6	Thorium-232	0.5	J	pCi/g
11182	125086	4.5	6	Uranium-234	0.6	J	pCi/g
11182	125087	4.5	6	Uranium-235/236	0.1	UJ	pCi/g
11182	125087	4.5	6	Uranium-238	0.7	J	pCi/g
11182	125086	4.5	6	Gross Alpha	10	UNV	pCi/g
11182	125086	4.5	6	Gross Beta	25	NV	pCi/g
11182	125086	4.5	6	Gross Beta	24	UNV	pCi/g
11182	125086	4.5	6	Thorium, Total	18	UNV	mg/kg
11182	125086	4.5	6	Technetium-99	1	UJ	pCi/g
11182	125078	6	7.5	Thorium, Total	18	UNV	mg/kg
11182	125078	6	7.5	Uranium, Total	11	UNV	mg/kg
11182	125079	7.5	9	Thorium, Total	18	UNV	mg/kg
11182	125079	7.5	9	Uranium, Total	11	UNV	mg/kg
11182	125080	9	10.5	Thorium, Total	18	UNV	mg/kg
11182	125080	9	10.5	Thorium, Total	18	UNV	mg/kg
11182	125081	9	10.5	Uranium, Total	11	UNV	mg/kg
11182	125081	9	10.5	Uranium, Total	11	UNV	mg/kg
11182	125084	10.5	12	Thorium, Total	18	UNV	mg/kg
11182	125084	10.5	12	Uranium, Total	11	UNV	mg/kg
11182	125085	12	13	Thorium, Total	18	UNV	mg/kg
11182	125085	12	13	Uranium, Total	11	UNV	mg/kg

APPENDIX B
TABLE B-1
HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11182	125083			Gross Alpha	0.005	UNV	pCi/g
11182	125083			Gross Beta	0.49	UNV	pCi/g
11183	125110	0	1.5	Uranium, Total	3.30	U	mg/kg
11183	125098	0	1.5	Gross Alpha	10.3	NV	pCi/g
11183	125110	0	1.5	Gross Beta	22.8	NV	pCi/g
11183	125110	0	1.5	Cesium-137	0.2	U	pCi/g
11183	125110	0	1.5	Gross Alpha	6.5	NV	pCi/g
11183	125110	0	1.5	Radium-226	0.9	J	pCi/g
11183	125110	0	1.5	Radium-228	1.4	-	pCi/g
11183	125110	0	1.5	Strontium-90	0.6	UJ	pCi/g
11183	125110	0	1.5	Thorium-228	0.7	J	pCi/g
11183	125110	0	1.5	Thorium-230	1.2	J	pCi/g
11183	125110	0	1.5	Thorium-232	0.7	J	pCi/g
11183	125110	0	1.5	Uranium-234	1.2	J	pCi/g
11183	125111	0	1.5	Uranium-235/236	0.1	UJ	pCi/g
11183	125111	0	1.5	Uranium-238	1.1	U	pCi/g
11183	125110	0	1.5	Gross Beta	25	NV	pCi/g
11183	125110	0	1.5	Thorium, Total	18	UNV	mg/kg
11183	125110	0	1.5	Technetium-99	1	U	pCi/g
11183	125099	1.5	3	Thorium, Total	18	UNV	mg/kg
11183	125099	1.5	3	Uranium, Total	11	UNV	mg/kg
11183	125100	3	4.5	Thorium, Total	18	UNV	mg/kg
11183	125100	3	4.5	Uranium, Total	11	UNV	mg/kg
11183	125101	4.5	6	Thorium, Total	18	UNV	mg/kg
11183	125101	4.5	6	Uranium, Total	11	UNV	mg/kg
11183	125102	6	7.5	Thorium, Total	18	UNV	mg/kg
11183	125102	6	7.5	Uranium, Total	11	UNV	mg/kg
11183	125103	7.5	9	Thorium, Total	18	UNV	mg/kg
11183	125103	7.5	9	Uranium, Total	11	UNV	mg/kg
11183	125104	9	10.5	Thorium, Total	18	UNV	mg/kg
11183	125104	9	10.5	Uranium, Total	11	UNV	mg/kg
11183	125105	10.5	12	Thorium, Total	18	UNV	mg/kg
11183	125105	10.5	12	Uranium, Total	11	UNV	mg/kg
11183	125106	12	13	Thorium, Total	18	UNV	mg/kg
11183	125106	12	13	Thorium, Total	18	UNV	mg/kg
11183	125107	12	13	Uranium, Total	11	UNV	mg/kg
11183	125107	12	13	Uranium, Total	11	UNV	mg/kg
11184	125032	0	1.5	Thorium, Total	18	UNV	mg/kg
11184	125032	0	1.5	Uranium, Total	11	UNV	mg/kg

APPENDIX B

TABLE B-1

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HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11184	125033	1.5	3	Thorium, Total	18	UNV	mg/kg
11184	125033	1.5	3	Uranium, Total	11	UNV	mg/kg
11184	125034	3	4.5	Thorium, Total	18	UNV	mg/kg
11184	125034	3	4.5	Uranium, Total	11	UNV	mg/kg
11184	125035	4.5	6	Thorium, Total	18	UNV	mg/kg
11184	125035	4.5	6	Thorium, Total	18	UNV	mg/kg
11184	125037	4.5	6	Uranium, Total	11	UNV	mg/kg
11184	125037	4.5	6	Uranium, Total	11	UNV	mg/kg
11184	125038	6	7.5	Thorium, Total	18	UNV	mg/kg
11184	125042	6	7.5	Uranium, Total	11	UNV	mg/kg
11184	125039	7.5	9	Uranium, Total	0.90	J	mg/kg
11184	125042	7.5	9	Gross Beta	13.1	NV	pCi/g
11184	125042	7.5	9	Radium-226	0.25	UJ	pCi/g
11184	125042	7.5	9	Cesium-137	0.2	U	pCi/g
11184	125042	7.5	9	Gross Alpha	5.2	NV	pCi/g
11184	125042	7.5	9	Radium-228	0.6	U	pCi/g
11184	125042	7.5	9	Strontium-90	0.5	UJ	pCi/g
11184	125042	7.5	9	Thorium-228	0.4	J	pCi/g
11184	125042	7.5	9	Thorium-230	0.8	J	pCi/g
11184	125042	7.5	9	Thorium-232	0.4	J	pCi/g
11184	125039	7.5	9	Uranium-234	0.4	J	pCi/g
11184	125040	7.5	9	Uranium-235/236	0.1	UJ	pCi/g
11184	125040	7.5	9	Uranium-238	0.3	J	pCi/g
11184	125042	7.5	9	Gross Alpha	12	UNV	pCi/g
11184	125042	7.5	9	Gross Beta	26	UNV	pCi/g
11184	125042	7.5	9	Thorium, Total	18	UNV	mg/kg
11184	125042	7.5	9	Technetium-99	1	UJ	pCi/g
11184	125041	9	10.5	Thorium, Total	18	UNV	mg/kg
11184	125041	9	10.5	Uranium, Total	11	UNV	mg/kg
11184	125036	10.5	12	Thorium, Total	18	UNV	mg/kg
11184	125036	10.5	12	Uranium, Total	11	UNV	mg/kg
11184	125066	12	13	Thorium, Total	18	UNV	mg/kg
11184	125066	12	13	Uranium, Total	11	UNV	mg/kg
11185	125055	0	1.5	Thorium, Total	18	UNV	mg/kg
11185	125055	0	1.5	Uranium, Total	11	UNV	mg/kg
11185	125056	1.5	3	Thorium, Total	18	UNV	mg/kg
11185	125056	1.5	3	Uranium, Total	11	UNV	mg/kg
11185	125057	3	4.5	Thorium, Total	18	UNV	mg/kg
11185	125057	3	4.5	Uranium, Total	11	UNV	mg/kg

APPENDIX B
TABLE B-1
HISTORICAL DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

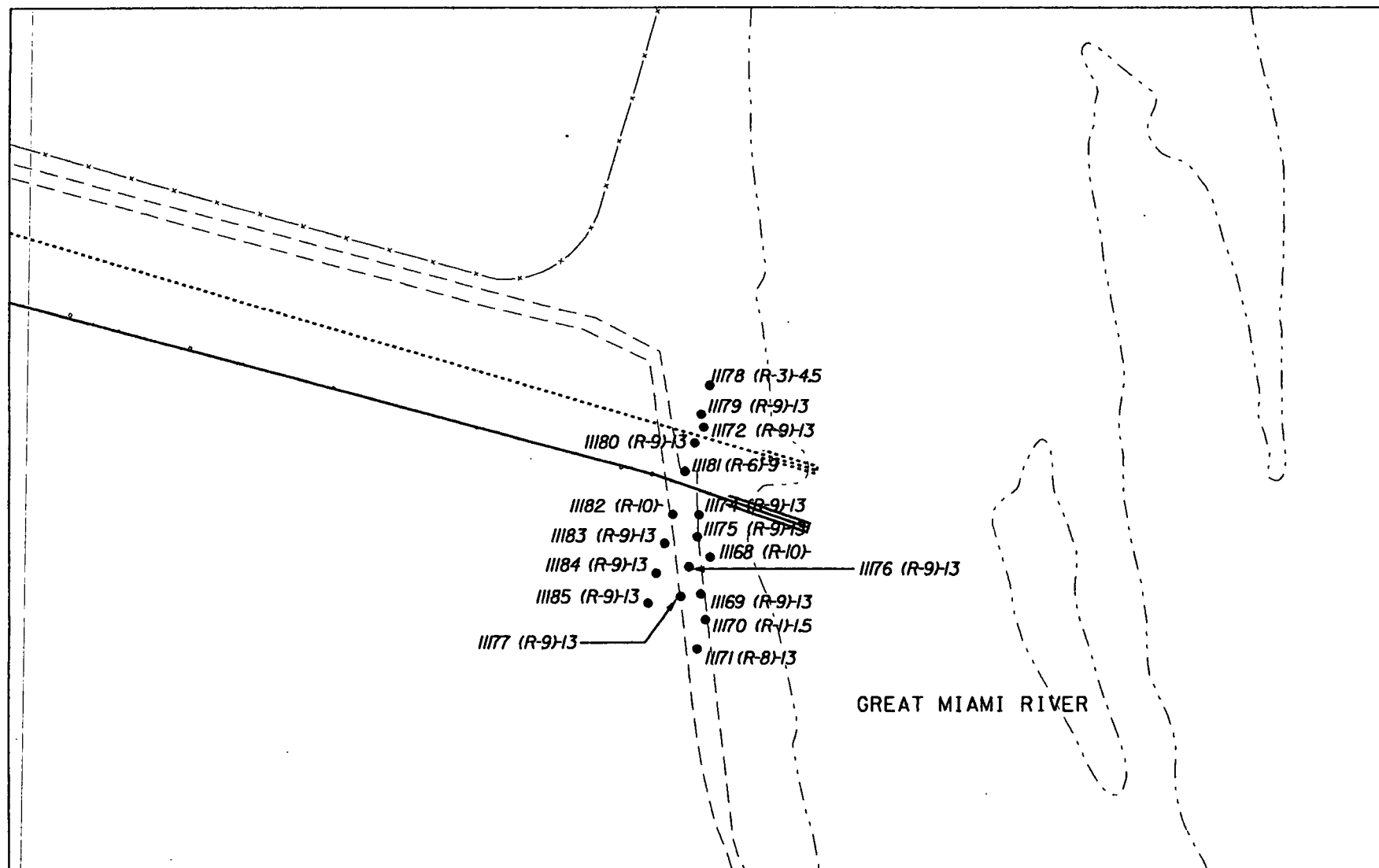
Boring ID	Sample ID	Top Depth (in feet)	Bottom Depth (in feet)	Parameter	Result	Qualifier	Units
11185	125058	4.5	6	Thorium, Total	18	UNV	mg/kg
11185	125058	4.5	6	Uranium, Total	11	UNV	mg/kg
11185	125059	6	7.5	Thorium, Total	18	UNV	mg/kg
11185	125059	6	7.5	Uranium, Total	11	UNV	mg/kg
11185	125060	7.5	9	Thorium, Total	18	UNV	mg/kg
11185	125060	7.5	9	Thorium, Total	18	UNV	mg/kg
11185	125062	7.5	9	Uranium, Total	11	UNV	mg/kg
11185	125062	7.5	9	Uranium, Total	11	UNV	mg/kg
11185	125063	9	10.5	Thorium, Total	18	UNV	mg/kg
11185	125065	9	10.5	Uranium, Total	11	UNV	mg/kg
11185	125065	10.5	12	Uranium, Total	0.60	J	mg/kg
11185	125065	10.5	12	Radium-226	0.25	UJ	pCi/g
11185	125065	10.5	12	Cesium-137	0.2	U	pCi/g
11185	125065	10.5	12	Gross Alpha	3.6	UNV	pCi/g
11185	125065	10.5	12	Gross Beta	5.4	UNV	pCi/g
11185	125065	10.5	12	Plutonium-238	0.2	UJ	pCi/g
11185	125065	10.5	12	Plutonium-239/240	0.2	UJ	pCi/g
11185	125065	10.5	12	Radium-228	0.6	U	pCi/g
11185	125065	10.5	12	Strontium-90	0.5	UJ	pCi/g
11185	125065	10.5	12	Thorium-228	0.1	UJ	pCi/g
11185	125065	10.5	12	Thorium-230	0.5	J	pCi/g
11185	125065	10.5	12	Thorium-232	0.2	J	pCi/g
11185	125065	10.5	12	Uranium-234	0.3	J	pCi/g
11185	125064	10.5	12	Uranium-235/236	0.1	UJ	pCi/g
11185	125064	10.5	12	Uranium-238	0.2	J	pCi/g
11185	125065	10.5	12	Thorium, Total	18	UNV	mg/kg
11185	125065	10.5	12	Technetium-99	1	UJ	pCi/g
11185	125061	12	13	Thorium, Total	18	UNV	mg/kg
11185	125061	12	13	Uranium, Total	11	UNV	mg/kg

APPENDIX B

TABLE B-2

PREDESIGN DATA FOR A9P3 ABANDONED OUTFALL DITCH PART TWO

Boring	Sample ID	Top	Bottom	Parameter	Result	Qual	Units
A9P3-GMRB-1	A9P3-GMRB-1^1-R	0	0.5	Cesium-137	0.256	-	pCi/g
A9P3-GMRB-1	A9P3-GMRB-1^1-R	0	0.5	Radium-226	0.609	-	pCi/g
A9P3-GMRB-1	A9P3-GMRB-1^1-R	0	0.5	Radium-228	0.562	-	pCi/g
A9P3-GMRB-1	A9P3-GMRB-1^1-R	0	0.5	Technetium-99	0.128	U	pCi/g
A9P3-GMRB-1	A9P3-GMRB-1^1-R	0	0.5	Thorium-228	0.589	-	pCi/g
A9P3-GMRB-1	A9P3-GMRB-1^1-R	0	0.5	Thorium-232	0.562	-	pCi/g
A9P3-GMRB-1	A9P3-GMRB-1^1-R	0	0.5	Uranium, Total	4.91	-	mg/kg
A9P3-GMRB-1	A9P3-GMRB-1^3-R	1	1.5	Thorium-232	0.567	-	pCi/g
A9P3-GMRB-1	A9P3-GMRB-1^3-R	1	1.5	Uranium, Total	5.41	-	mg/kg
A9P3-GMRB-1	A9P3-GMRB-1^6-R	2.5	3	Thorium-232	0.523	-	pCi/g
A9P3-GMRB-1	A9P3-GMRB-1^6-R	2.5	3	Uranium, Total	2.79	-	mg/kg
A9P3-GMRB-2	A9P3-GMRB-2^12-R	5.5	6	Thorium-232	0.618	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^15-R	7	7.5	Thorium-232	0.626	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^18-R	8.5	9	Thorium-232	0.564	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^1-R	0	0.5	Cesium-137	0.0685	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^1-R	0	0.5	Radium-226	0.579	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^1-R	0	0.5	Radium-228	0.452	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^1-R	0	0.5	Technetium-99	0.137	U	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^1-R	0	0.5	Thorium-228	0.449	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^1-R	0	0.5	Thorium-232	0.452	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^1-R	0	0.5	Uranium, Total	4.28	-	mg/kg
A9P3-GMRB-2	A9P3-GMRB-2^21-R	10	10.5	Thorium-232	0.57	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^24-R	11.5	12	Thorium-232	0.494	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^26-R	12.5	13	Thorium-232	0.433	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^3-R	1	1.5	Thorium-232	0.612	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^6-R	2.5	3	Thorium-232	0.542	-	pCi/g
A9P3-GMRB-2	A9P3-GMRB-2^9-R	4	4.5	Thorium-232	0.451	-	pCi/g
A9P3-GMRB-3	A9P3-GMRB-3^1-R	0	0.5	Cesium-137	0	U	pCi/g
A9P3-GMRB-3	A9P3-GMRB-3^1-R	0	0.5	Radium-226	0.635	-	pCi/g
A9P3-GMRB-3	A9P3-GMRB-3^1-R	0	0.5	Radium-228	0.251	-	pCi/g
A9P3-GMRB-3	A9P3-GMRB-3^1-R	0	0.5	Technetium-99	0.0245	U	pCi/g
A9P3-GMRB-3	A9P3-GMRB-3^1-R	0	0.5	Thorium-228	0.252	-	pCi/g
A9P3-GMRB-3	A9P3-GMRB-3^1-R	0	0.5	Thorium-232	0.251	-	pCi/g
A9P3-GMRB-3	A9P3-GMRB-3^1-R	0	0.5	Uranium, Total	1.97	J	mg/kg
A9P3-GMRB-3	A9P3-GMRB-3^26-R	12.5	13	Thorium-232	0.532	-	pCi/g
A9P3-GMRB-4	A9P3-GMRB-4^1-R	0	0.5	Cesium-137	0	U	pCi/g
A9P3-GMRB-4	A9P3-GMRB-4^1-R	0	0.5	Radium-226	0.414	-	pCi/g
A9P3-GMRB-4	A9P3-GMRB-4^1-R	0	0.5	Radium-228	0.231	-	pCi/g
A9P3-GMRB-4	A9P3-GMRB-4^1-R	0	0.5	Technetium-99	0.0574	U	pCi/g
A9P3-GMRB-4	A9P3-GMRB-4^1-R	0	0.5	Thorium-228	0.228	-	pCi/g
A9P3-GMRB-4	A9P3-GMRB-4^1-R	0	0.5	Thorium-232	0.231	-	pCi/g
A9P3-GMRB-4	A9P3-GMRB-4^1-R	0	0.5	Uranium, Total	2.13	J	mg/kg
A9P3-GMRB-4	A9P3-GMRB-4^3-R	1	1.5	Uranium, Total	2.56	-	mg/kg



LEGEND:

ABANDONED 16"
OUTFALL LINE

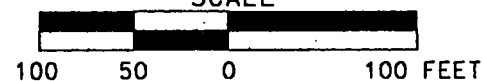
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EXISTING 24"
OUTFALL LINE

●

HISTORICAL SAMPLE
LOCATION

SCALE



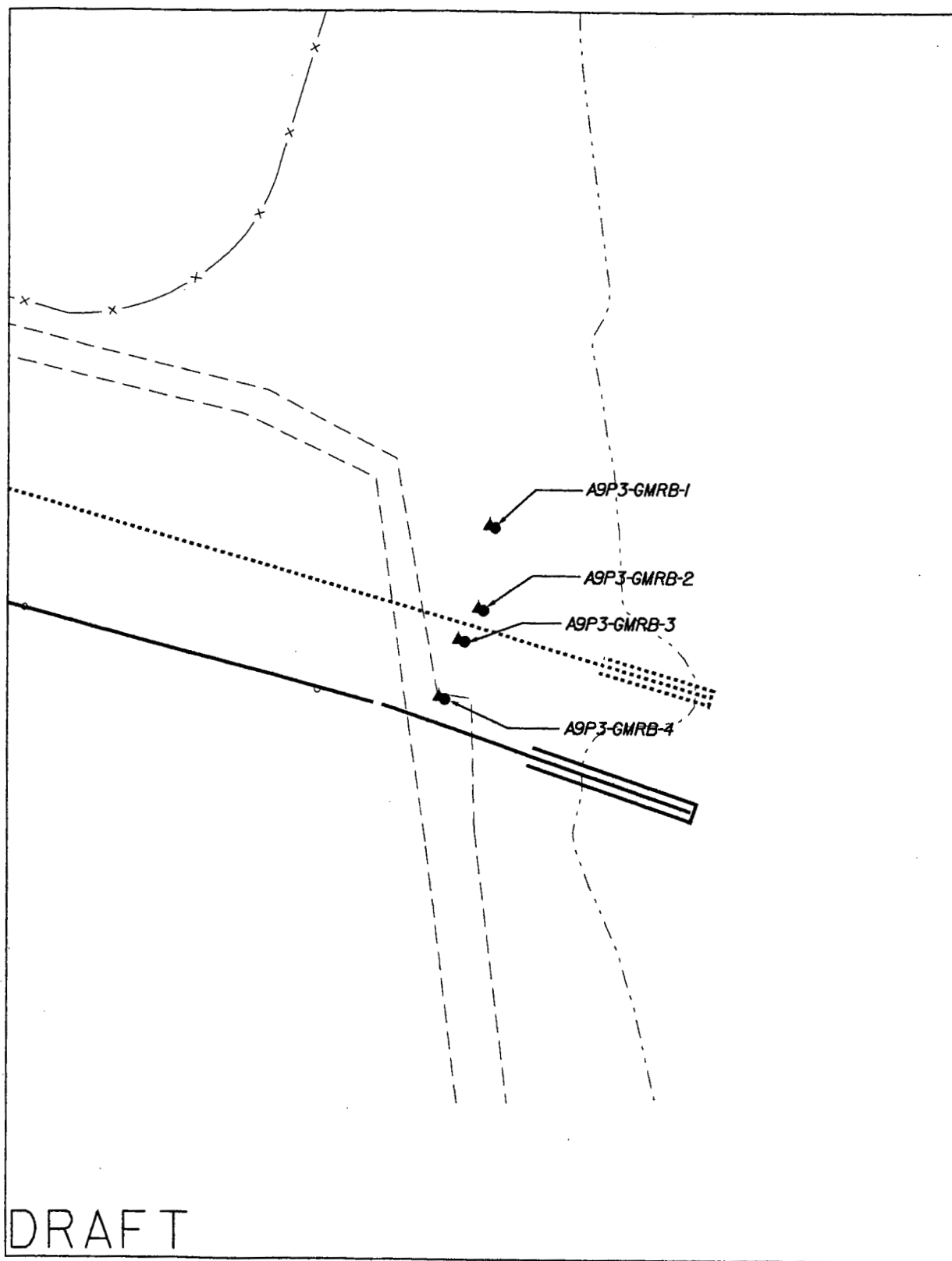
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FIGURE B1-1. HISTORICAL SAMPLE LOCATIONS FOR
THE GREAT MIAMI RIVER BANK AREA

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STATE PLANNING COORDINATE SYSTEM 1983

25-AUG-2004



DRAFT

LEGEND:

- BORING LOCATION
- ▲ HISTORICAL ABOVE-FRL BORING

- OLD OUTFALL LINE
- NEW OUTFALL LINE

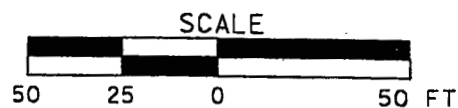


FIGURE B2-1. PREDESIGN SAMPLE LOCATIONS FOR
THE GREAT MIAMI RIVER BANK AREA

APPENDIX C
GEOTECHNICAL DATA

TABLE C-1
Geotechnical Data

Boring No. GT	Sample ID	Recovery Depth (ft)	Passing #4 4.75 mm	Percent Gravel (+#4)	Percent Sand (#4-#200)	Passing #200 0.75 mm	Liquid Limit	Plastic Limit	Plasticity Index	Classification
1	6ft	6 - 7.5	100	0.0	66.8	33.2	Non-Plastic			SM-Silty Sand
1	9ft	9 - 10	100	0.0	93.5	6.5	—	—	—	SP-SM Poorly graded sand w/silt
2	5ft	5 - 6.5	100	0.0	14.5	85.5	—	—	—	<i>CL-Lean Clay</i>
2	7ft	6.75 - 8	100	0.0	11.9	88.1	41	19	22	<i>CL-Lean Clay</i>
2	8ft	8 - 9	95.8	4.2	27.6	68.2	38	19	19	<i>CL-Lean Clay w/sand</i>
2	9ft	9 - 10.25	100	0.0	20.5	79.5	35	19	16	<i>CL-Lean Clay w/sand</i>
2	10.5ft	10.5 - 11.5	100	0.0	24.8	75.2	—	—	—	<i>CL-Lean Clay w/sand</i>
2	11.5ft	11.5 - 12	100	0.0	48.1	51.9	—	—	—	<i>ML-Sandy Silt</i>
3	4ft	4 - 5.5	100	0.0	23.7	76.3	31	18	13	<i>CL-Lean Clay w/sand</i>
3	6.5ft	6.5 - 7	100	0.0	54.0	46.0	—	—	—	<i>SM-Silty Sand</i>
3	8ft	8 - 9.5	98.4	1.6	86.0	12.4	—	—	—	<i>SM-Silty Sand</i>
3	11ft	10 - 12	36.7	63.3	32.8	3.9	—	—	—	GW-Well Graded Gravel w/sand
4A	5ft	4.5 - 5.5	100.0	0.0	17.6	82.4	28	19	9	<i>CL-Lean Clay w/sand</i>
4A	7ft	6.25 - 7	99.6	0.4	72.9	26.7	—	—	—	<i>SM-Silty Sand</i>
4A	8ft	8 - 8.25	96.1	3.9	70.9	25.2	—	—	—	<i>SM-Silty Sand</i>
4A	8.5ft	8.25 - 8.75	93.7	6.3	86.6	7.1	—	—	—	<i>SM-Silty Sand</i>
4A	9ft	8.75 - 9	93.8	6.2	71.2	22.6	—	—	—	<i>SM-Silty Sand</i>
4A	10ft	9 - 10	48.2	51.8	44.3	3.9	—	—	—	GW-Well Graded Gravel w/sand
5A	4ft	3 - 4.5	100.0	0.0	51.4	48.6	24	19	5	SC-SM Silty Clayey Sand
5A	8ft	7.5 - 8.5	100.0	0.0	23.5	76.5	29	20	9	<i>CL-Lean Clay w/sand</i>
5A	11ft	10.5-11.25	100.0	0.0	42.3	57.7	—	—	—	<i>ML-Sandy Silt</i>
5A	13ft	11.25 - 13	46.5	53.5	41.3	5.2	—	—	—	GW-Well Graded Gravel w/sand
6A	4ft	3.5 - 4.75	100.0	0.0	35.7	64.3	27	20	7	<i>CL-ML Silty Clay</i>
6A	7ft	6.5 - 7.5	99.8	0.2	31.6	68.2	Non-Plastic		—	<i>ML-Sandy Silt</i>
6A	8ft	7.75 - 8.75	100.0	0.0	23.2	76.8	—	—	—	<i>ML-Silt w/sand</i>
6A	9ft	8.5 - 9	100.0	0.0	52.0	48.0	—	—	—	<i>SM-Silty Sand</i>
6A	12ft	11.5 - 12.5	100.0	0.0	30.0	70.0	25	20	5	<i>CL-ML Silty Clay</i>
6A	13.5ft	13.5-13.75	100.0	0.0	74.1	25.9	—	—	—	<i>SM-Silty Sand</i>

Classifications in *Italics* are estimated from visual observation.

REMEDIATION OF ABANDONED OUTFALL LINE

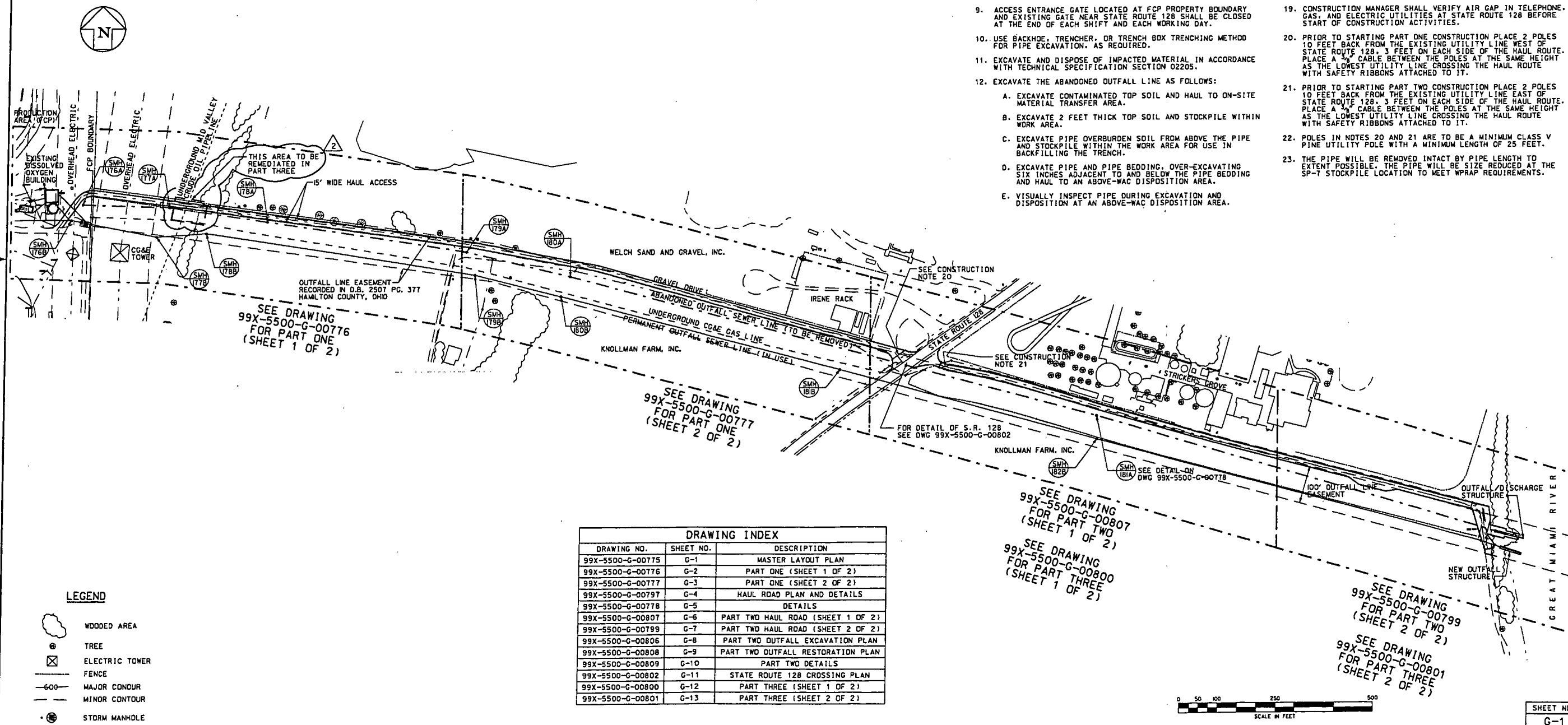
5670

GENERAL NOTES

1. HORIZONTAL CONTROLS SHOWN ARE BASED UPON NORTH AMERICAN DATUM 1983 (NAD 83).
2. VERTICAL CONTROLS SHOWN ARE BASED UPON NATIONAL GEODETIC VERTICAL DATUM 1929 (NGVD 29).
3. DIMENSIONS SHOWN TAKE PRECEDENCE OVER SCALED DIMENSIONS.
4. PROTECT UNDERGROUND AND OVERHEAD UTILITIES DURING EXCAVATION OF THE ABANDONED OUTFALL LINE.
5. TECHNICAL SPECIFICATIONS GOVERNING THIS ACTIVITY ARE THE LATEST REVISION OF TECHNICAL SPECIFICATIONS FOR SOIL AND DISPOSAL FACILITY PROJECT - EXCAVATION FOR REMEDIATION (DOCUMENT NO. 20300-TS-0001) AND OSDF PHASE V TECHNICAL SPECIFICATIONS (DOCUMENT NO. 20105-TS-0001).
6. EXISTING SURFACE ELEVATIONS WILL NOT BE RAISED WITHIN THE 100 YEAR FLOOD PLAIN OF THE GREAT MIAMI RIVER.

CONSTRUCTION NOTES

1. GALVANIZED WOVEN WIRE FABRIC CONSTRUCTION FENCE MATERIAL AND INSTALLATION SHALL BE IN ACCORDANCE WITH OSDF TECHNICAL SPECIFICATION 02200. LOCATE CONSTRUCTION FENCE AS SHOWN ON THE DRAWINGS.
2. SOIL EROSION AND SEDIMENT CONTROL MATERIAL AND INSTALLATION SHALL BE IN ACCORDANCE WITH OSDF TECHNICAL SPECIFICATION 02270.
3. BEFORE START OF EXCAVATION, PLUG DOWNSTREAM SIDE OF MANHOLE SMH181A WITH MECHANICAL PLUG AND CONCRETE TO CONTROL CONSTRUCTION WATER FROM UPSTREAM PHASE I EXCAVATION AND PREVENT BACKFLOW FROM GREAT MIAMI RIVER. SEE DRAWING FOR PLUG DETAIL.
4. ALL UTILITIES SHALL BE LOCATED IN THE FIELD PRIOR TO SITE PREPARATION ACTIVITIES.
5. ESTABLISH CONSTRUCTION SIGNAGE IN APPROPRIATE LOCATION. SIGNAGE IN RIGHT-OF-WAY OF STATE ROUTE 128 SHALL BE IN ACCORDANCE WITH THE OHIO DEPARTMENT OF TRANSPORTATION.
6. IMPROVE EXISTING HAUL ROAD ROUTE ADJACENT TO TRENCHING AREA AS SHOWN ON DRAWINGS.
7. PROTECT MID VALLEY PIPELINE COMPANY CRUDE OIL PIPELINE WITH RAMP AS SHOWN ON DRAWINGS.
8. EXISTING UNDERGROUND TELEPHONE CONDUIT AT TRENCH CROSSING MAY BE REMOVED AND REPLACED AS NECESSARY.
9. ACCESS ENTRANCE GATE LOCATED AT FCP PROPERTY BOUNDARY AND EXISTING GATE NEAR STATE ROUTE 128 SHALL BE CLOSED AT THE END OF EACH SHIFT AND EACH WORKING DAY.
10. USE BACKHOE, TRENCHER, OR TRENCH BOX TRENCHING METHOD FOR PIPE EXCAVATION, AS REQUIRED.
11. EXCAVATE AND DISPOSE OF IMPACTED MATERIAL IN ACCORDANCE WITH TECHNICAL SPECIFICATION SECTION 02205.
12. EXCAVATE THE ABANDONED OUTFALL LINE AS FOLLOWS:
 - A. EXCAVATE CONTAMINATED TOP SOIL AND HAUL TO ON-SITE MATERIAL TRANSFER AREA.
 - B. EXCAVATE 2 FEET THICK TOP SOIL AND STOCKPILE WITHIN WORK AREA.
 - C. EXCAVATE PIPE OVERBURDEN SOIL FROM ABOVE THE PIPE AND WITHIN THE WORK AREA FOR USE IN BACKFILLING THE TRENCH.
 - D. EXCAVATE PIPE AND PIPE BEDDING, OVER-EXCAVATING SIX INCHES ADJACENT TO AND BELOW THE PIPE BEDDING AND HAUL TO AN ABOVE-WAC DISPOSITION AREA.
 - E. VISUALLY INSPECT PIPE DURING EXCAVATION AND DISPOSITION AT AN ABOVE-WAC DISPOSITION AREA.
13. F. AFTER EXCAVATION, PERFORM REAL TIME AND CERTIFICATION SAMPLING AT APPROXIMATELY 25 FOOT INTERVALS ALONG THE TRENCH BOTTOM. BASED ON RESULTS FROM REAL TIME SAMPLING, IF TRENCH BOTTOM FOUND CONTAMINATED, PERFORM SUPPLEMENTAL OVER-EXCAVATION AND PERFORM REAL TIME AND CERTIFICATION SAMPLING.
14. G. USING STOCKPILED OVERBURDEN SOIL AND TOP SOIL, BACKFILL THE TRENCH EXCAVATION AS SHOWN IN TYPICAL TRENCH DETAIL.
15. DO NOT EXCAVATE ABANDONED OUTFALL LINE OVERBURDEN SOIL DIRECTLY BELOW THE MIDVALLEY OIL PIPELINE SHOWN.
16. DO NOT STAGE CONSTRUCTION EQUIPMENT WITHIN 10 FEET OF THE TOP OF THE TRENCH EXCAVATION.
17. PERFORM SUPPLEMENTAL OVEREXCAVATION AS DIRECTED BY THE CONSTRUCTION MANAGER.
18. DISTURBED AREAS WILL BE RESTORED TO ORIGINAL ELEVATIONS PRIOR TO SEEDING ACTIVITIES.
19. VEGETATE THE DISTURBED AREAS USING SEED MIX AS APPROVED BY PROPERTY LAND OWNER.
20. GROUT USED TO SEAL ABANDONED OUTFALL LINE IN DESIGNATED AREAS SHALL BE IN ACCORDANCE WITH ODOT 705.22 OR APPROVED EQUAL.
21. CONSTRUCTION MANAGER SHALL VERIFY AIR GAP IN TELEPHONE, GAS, AND ELECTRIC UTILITIES AT STATE ROUTE 128 BEFORE START OF CONSTRUCTION ACTIVITIES.
22. PRIOR TO STARTING PART ONE CONSTRUCTION PLACE 2 POLES 10 FEET BACK FROM THE EXISTING UTILITY LINE WEST OF STATE ROUTE 128, 3 FEET ON EACH SIDE OF THE HAUL ROUTE. PLACE A 2" CABLE BETWEEN THE POLES AT THE SAME HEIGHT AS THE LOWEST UTILITY LINE CROSSING THE HAUL ROUTE WITH SAFETY RIBBONS ATTACHED TO IT.
23. PRIOR TO STARTING PART TWO CONSTRUCTION PLACE 2 POLES 10 FEET BACK FROM THE EXISTING UTILITY LINE EAST OF STATE ROUTE 128, 3 FEET ON EACH SIDE OF THE HAUL ROUTE. PLACE A 2" CABLE BETWEEN THE POLES AT THE SAME HEIGHT AS THE LOWEST UTILITY LINE CROSSING THE HAUL ROUTE WITH SAFETY RIBBONS ATTACHED TO IT.
24. POLES IN NOTES 20 AND 21 ARE TO BE A MINIMUM CLASS V PINE UTILITY POLE WITH A MINIMUM LENGTH OF 25 FEET.
25. THE PIPE WILL BE REMOVED INTACT BY PIPE LENGTH TO EXTENT POSSIBLE. THE PIPE WILL BE SIZE REDUCED AT THE SP-7 STOCKPILE LOCATION TO MEET WPAIP REQUIREMENTS.



DRAWING INDEX		
DRAWING NO.	SHEET NO.	DESCRIPTION
99X-5500-G-00775	G-1	MASTER LAYOUT PLAN
99X-5500-G-00776	G-2	PART ONE (SHEET 1 OF 2)
99X-5500-G-00777	G-3	PART ONE (SHEET 2 OF 2)
99X-5500-G-00797	G-4	HAUL ROAD PLAN AND DETAILS
99X-5500-G-00778	G-5	DETAILS
99X-5500-G-00807	G-6	PART TWO HAUL ROAD (SHEET 1 OF 2)
99X-5500-G-00799	G-7	PART TWO HAUL ROAD (SHEET 2 OF 2)
99X-5500-G-00806	G-8	PART TWO OUTFALL EXCAVATION PLAN
99X-5500-G-00808	G-9	PART TWO OUTFALL RESTORATION PLAN
99X-5500-G-00809	G-10	PART TWO DETAILS
99X-5500-G-00802	G-11	STATE ROUTE 128 CROSSING PLAN
99X-5500-G-00800	G-12	PART THREE (SHEET 1 OF 2)
99X-5500-G-00801	G-13	PART THREE (SHEET 2 OF 2)

LEGEND

- WOODED AREA
- TREE
- ELECTRIC TOWER
- FENCE
- MAJOR CONTOUR
- MINOR CONTOUR
- STORM MANHOLE
- VALVE
- FIRE HYDRANT
- RAILROAD
- EASEMENT LINE

NO.	REVISIONS	DATE	OWN.	BY	APPD.	NO.	REVISIONS	DATE	OWN.	BY	APPD.	REF. DWG. NO.
	2											

NOTE:
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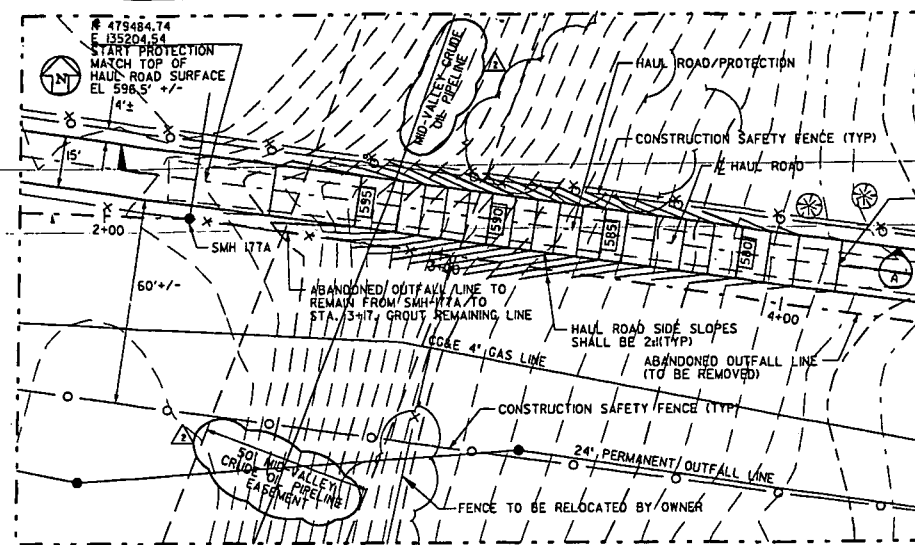
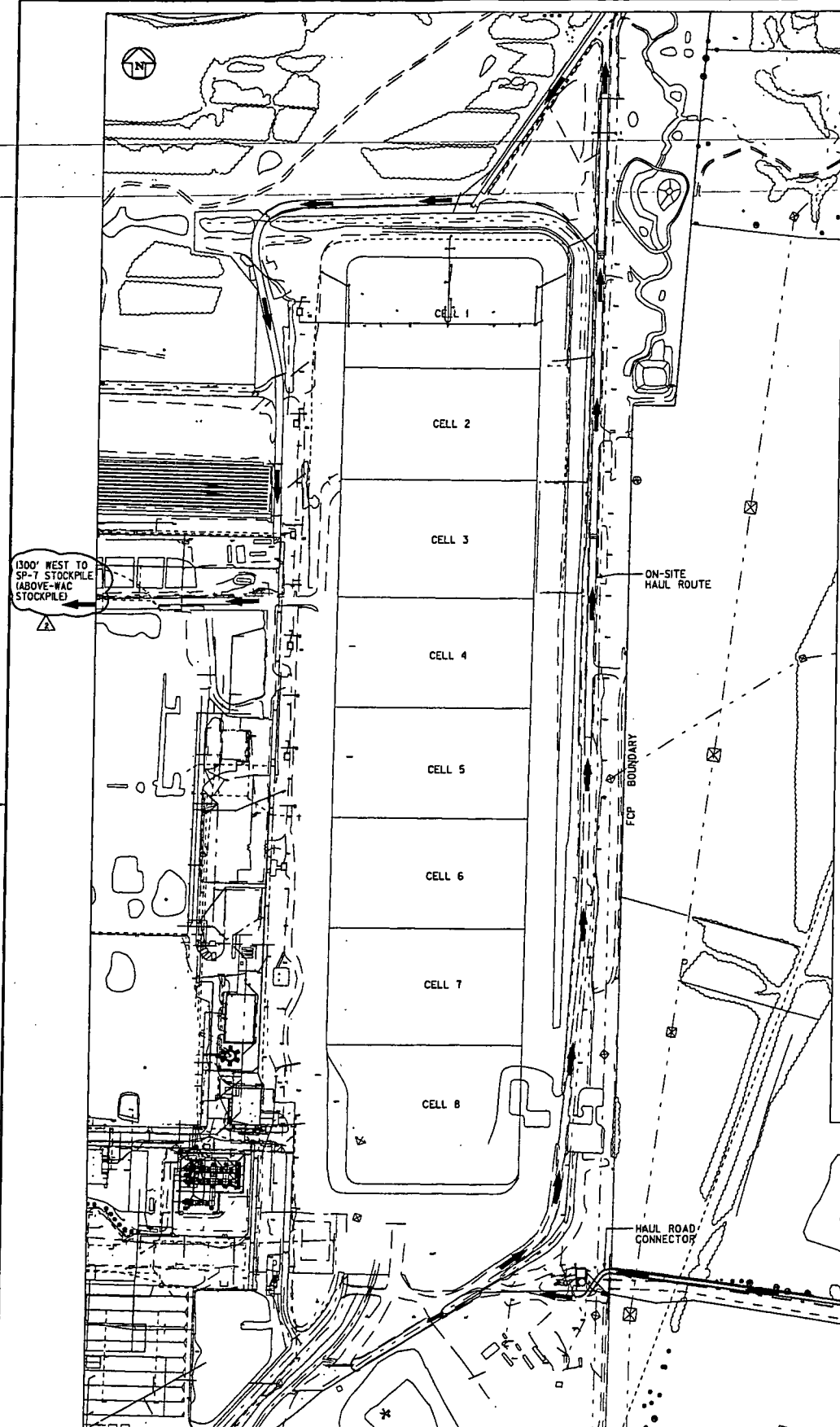
CONFIGURATION
MANAGEMENT
DRAWING
DATE: 8/30/2004
C. VAN ARMAN
COORDINATOR ENGINEER

APPROVALS			
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ELECTRICAL	11/20/04	ENVIRONMENTAL	11/20/04
ENGINEER	11/20/04	WASTE MGMT.	11/20/04
INSTRUMENT	11/20/04	SECURITY	11/20/04
MECHANICAL	11/20/04	PROJECTS	11/20/04
WAO	11/20/04	CONSTRUCTION	11/20/04
CHECKED	11/20/04	CHARACTERIZATION	11/20/04
APPROVED	11/20/04	QA	11/20/04

Fernald Closure Project
FLUOR FERNALD, INC.
U.S. DEPARTMENT OF ENERGY

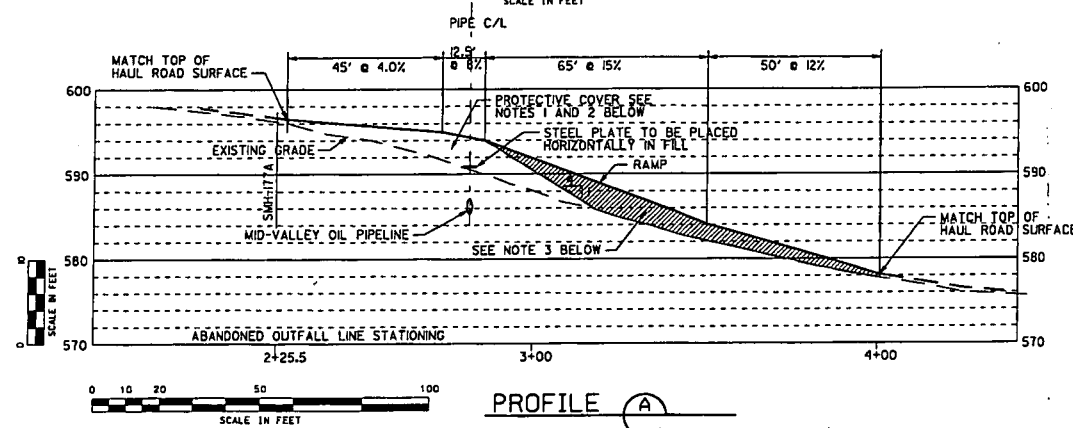
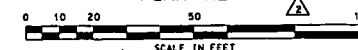
AREA 9 PHASE III
REMEDIATION OF ABANDONED OUTFALL LINE
MASTER LAYOUT PLAN
PROJECT 2020
DATE 8/30/2004
DRAWN RAL LANGRISH
99X-5500-G-00775 2A

FILE NAME: /osdf/Project2020/99xg0775.dgn



PROTECTION OVER MID-VALLEY CRUDE OIL PIPELINE

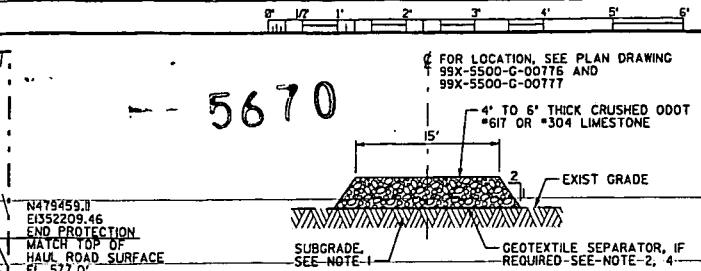
PLAN VIEW



PROFILE A

NOTES:

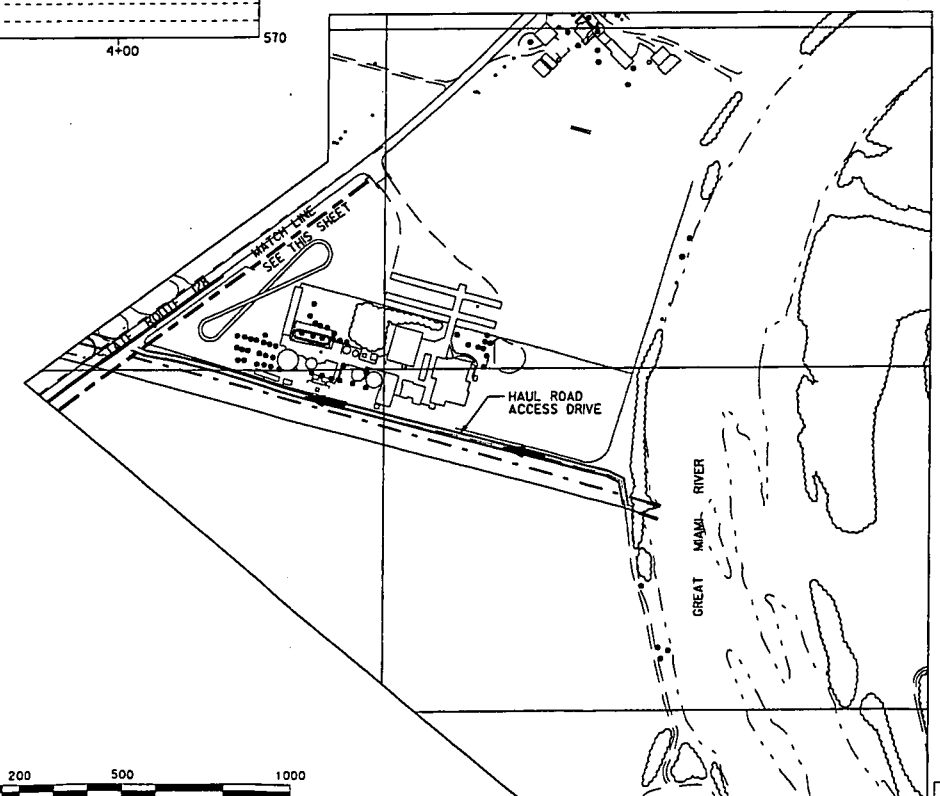
1. PRIOR TO TRENCH EXCAVATION, CONSTRUCT PROTECTIVE COVER OVER MID-VALLEY CRUDE OIL PIPELINE. PROTECTIVE COVER SHALL BE CONSTRUCTED IN ACCORDANCE WITH MID-VALLEY PIPELINE COMPANY REQUIREMENTS.
2. FILL MATERIAL FOR PROTECTIVE COVER SHALL BE #57 CRUSHED ANGULAR STONE UNLESS APPROVED OTHERWISE BY MID-VALLEY PIPELINE COMPANY.
3. CONSTRUCT RAMP AFTER REMOVAL OF ABANDONED OUTFALL LINE AND BACKFILL OF TRENCH FROM APPROXIMATELY STATION 3+17 TO STATION 4+00.



TYPICAL HAUL ROAD CROSS SECTION

NOTES:

1. USE ODOT TYPE 'D' DUMPED ROCK OR AASHTO #2 STONE TO IMPROVE UNSUITABLE EXISTING SUBGRADE.
2. GEOTEXTILE SEPARATOR SHALL BE IN ACCORDANCE WITH OSDF TECHNICAL SPECIFICATION 02714.
3. CROSS SECTION APPLIES TO HAUL ROAD ACCESS DRIVE, CONNECTOR, AND TRUCK TURN AROUND.
4. GEOTEXTILE SEPARATOR MAY BE USED IN AREAS WHEN WARRANTED BY SUBGRADE CONDITIONS.



SHEET NO.

G-4

NO.	REVISIONS	DATE	OWN.	BY	APPD.	NO.	REVISIONS	DATE	OWN.	BY	APPD.	REF.	OWG.	NO.

NOTE:
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MANUALLY.

CONFIGURATION
DRAWING
DATE: 6/20/04
CHECKED: [Signature]
APPROVED: [Signature]

APPROVALS	DATE	BY	APPD.
CIVIL & STR. ENGR.	6/20/04	[Signature]	6/20/04
ELECTRICAL ENGINEER	6/20/04	[Signature]	6/20/04
MECHANICAL ENGINEER	6/20/04	[Signature]	6/20/04
INSTRUMENTATION ENGINEER	6/20/04	[Signature]	6/20/04
WAD	6/20/04	[Signature]	6/20/04
CHECKED	6/20/04	[Signature]	6/20/04
APPROVED	6/20/04	[Signature]	6/20/04

Fernald Closure Project

FLUOR FERNALD, INC.

U.S. DEPARTMENT OF ENERGY

AREA 9 PHASE III
REMEDATION OF ABANDONED OUTFALL LINE
HAUL ROAD PLAN AND DETAIL

PROJECT 200
DATE: 8/27/2004
DRAWN: RLM, LINDGREN
99X-5500-G-00797 2A

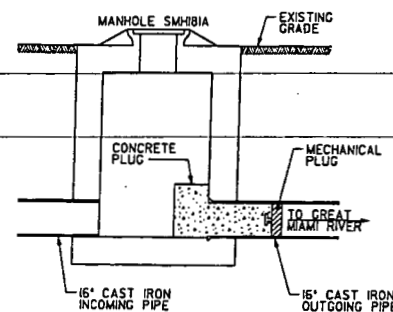
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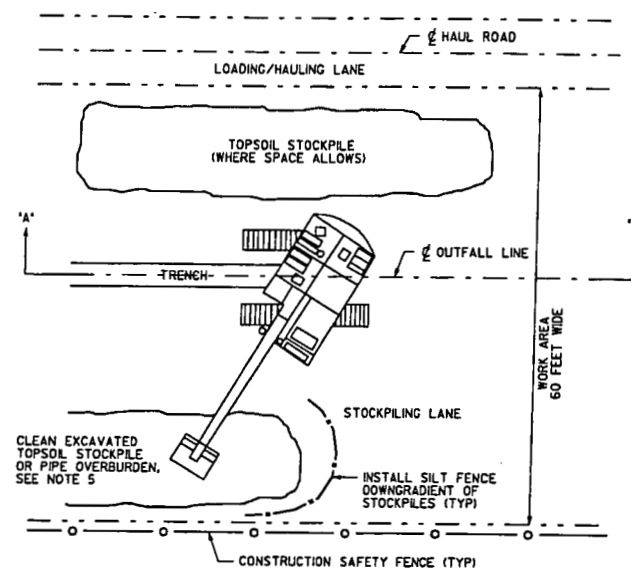
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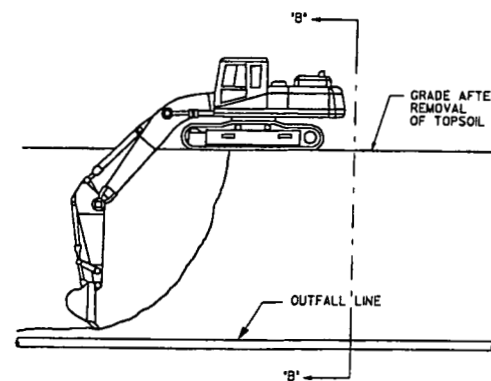
1. AFTER PRELIMINARY ANALYSIS OF SOIL SAMPLES AND REAL-TIME SCANNING OF THE EXCAVATION HAS BEEN PERFORMED, THE SOIL STOCKPILED WITHIN THE WORK AREA (NOT INCLUDING TOPSOIL) WILL BE USED AS BACKFILL FOR THE TRENCH EXCAVATION AND COMPACTED AT DEPTHS BELOW 5 TO 6 FEET. ADDITIONAL TRENCH BACKFILL REQUIRED TO REPLACE REMOVED MATERIAL WILL BE PURCHASED FROM A COMMERCIAL VENDOR.
2. THE STOCKPILED TOPSOIL WILL BE BACKFILLED IN THE TOP TWO FOOT OF THE WORK AREA. ADDITIONAL TOPSOIL AS REQUIRED WILL BE PURCHASED FROM A COMMERCIAL VENDOR. TOPSOIL WILL BE SPREAD AND ROLLED USING THE CONSTRUCTION EQUIPMENT WITH NO COMPACTION IN ACCORDANCE WITH THE LAND OWNERS REQUIREMENTS. TOPSOIL WILL BE GRADED TO MATCH THE EXISTING ADJACENT GRADES.
3. ALL AREAS OUTSIDE THE CROP AREA SHALL BE RESTORED TO ORIGINAL CONDITIONS.
4. OVEREXCAVATE 6 INCHES OF SOIL BELOW AND ALONG SIDES OF BEDDING MATERIAL AND HAUL TO SP-7 STOCKPILE AREA.
5. EXCAVATE TOP 6 INCHES OF TOPSOIL AT LOCATION OF PIPE OVERBURDEN STOCKPILE, REPLACE TOPSOIL WHEN STOCKPILE IS DEPLETED.



MECHANICAL PLUG
NOT TO SCALE



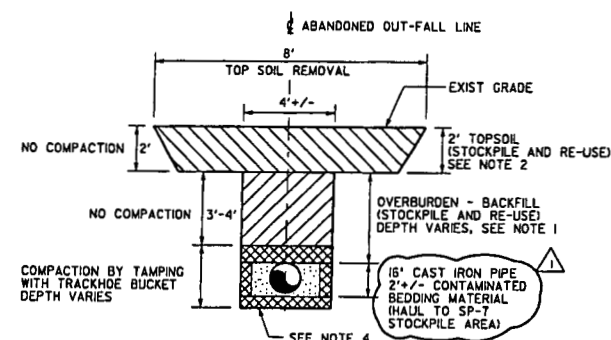
SECTION 'A-A'



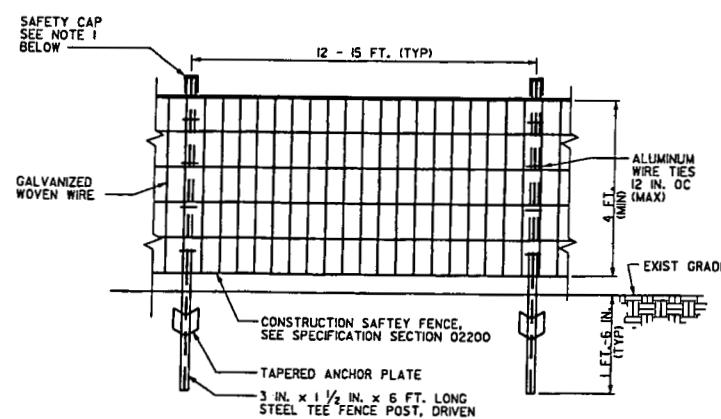
TYPICAL WORKING AREA DETAIL
NOT TO SCALE

NOTES:

1. A TRENCH BOX MAY BE USED AS NECESSARY.



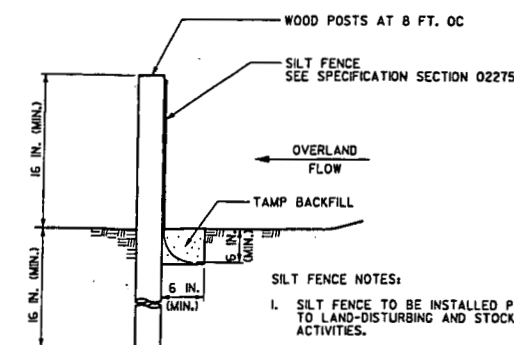
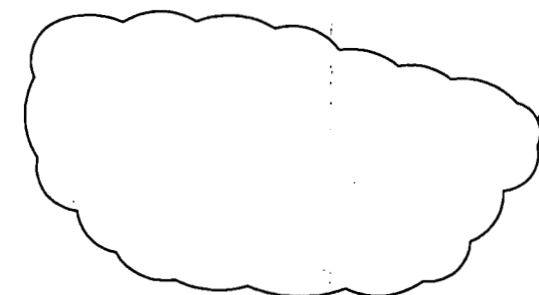
TYPICAL TRENCHING AND BACKFILL DETAIL
NOT TO SCALE



NOTES:

1. INSTALL SAFETY CAPS ON T-POSTS THAT ARE LESS THAN 4 FEET IN HEIGHT ABOVE-GRADE.

CONSTRUCTION SAFETY FENCE DETAIL
NOT TO SCALE



SILT FENCE NOTES:

1. SILT FENCE TO BE INSTALLED PRIOR TO LAND-DISTURBING AND STOCKPILING ACTIVITIES.
2. WOOD POSTS SHALL BE 2 IN. X 2 IN. (MIN.) OF OAK OR SIMILAR HARDWOOD.
3. BACKFILL MATERIAL IN ACCORDANCE WITH SPECIFICATION SECTION 02200.

SILT FENCE DETAIL
NOT TO SCALE

NO.	REVISIONS	DATE/DWN.	BY/APPD.	NO.	REVISIONS	DATE/DWN.	BY/APPD.	REF. DWG. NO.
1	REVISE NOTE 4, REVISE TRENCHING AND BACKFILL DETAIL, REMOVE PIPE GROUTING DETAIL.	1/2/04	RML	GEP				
0	ISSUED CERTIFIED FOR CONSTRUCTION	6/2/04	RML	GEP				

NOTE:
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CONFIGURATION
MANAGEMENT
DRAWING

C. Y. H. MOORE
COORDINATING ENGINEER
DATE
6/2/04

APPROVALS	DATE
CIVIL & STR. L. SHEP	6/2/04
ELECTRICAL ENGINEER	6/2/04
INSTRUMENT MECHANICAL	6/2/04
WASTE MANAGEMENT	6/2/04
SAFETY ENGINEER	6/2/04
MAINTENANCE	6/2/04
PROJECT MANAGER	6/2/04
SECURITY	6/2/04
CONSTRUCTION	6/2/04
QUALITY CONTROL	6/2/04

Fernald Closure Project
FLUOR FERNALD, INC.
U.S. DEPARTMENT OF ENERGY

AREA 9 PHASE III
REMEDIATION OF ABANDONED OUTFALL LINE
DETAILS
PROJECT: 2020
DATE: 6/20/2004
DRAWN: R. M. LINDEN
99X-5500-G-00778

SHEET NO.
G-5

FILE NAME: /osdf/Project2020/99xg0778.dgn

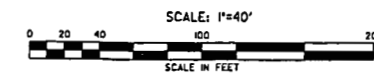
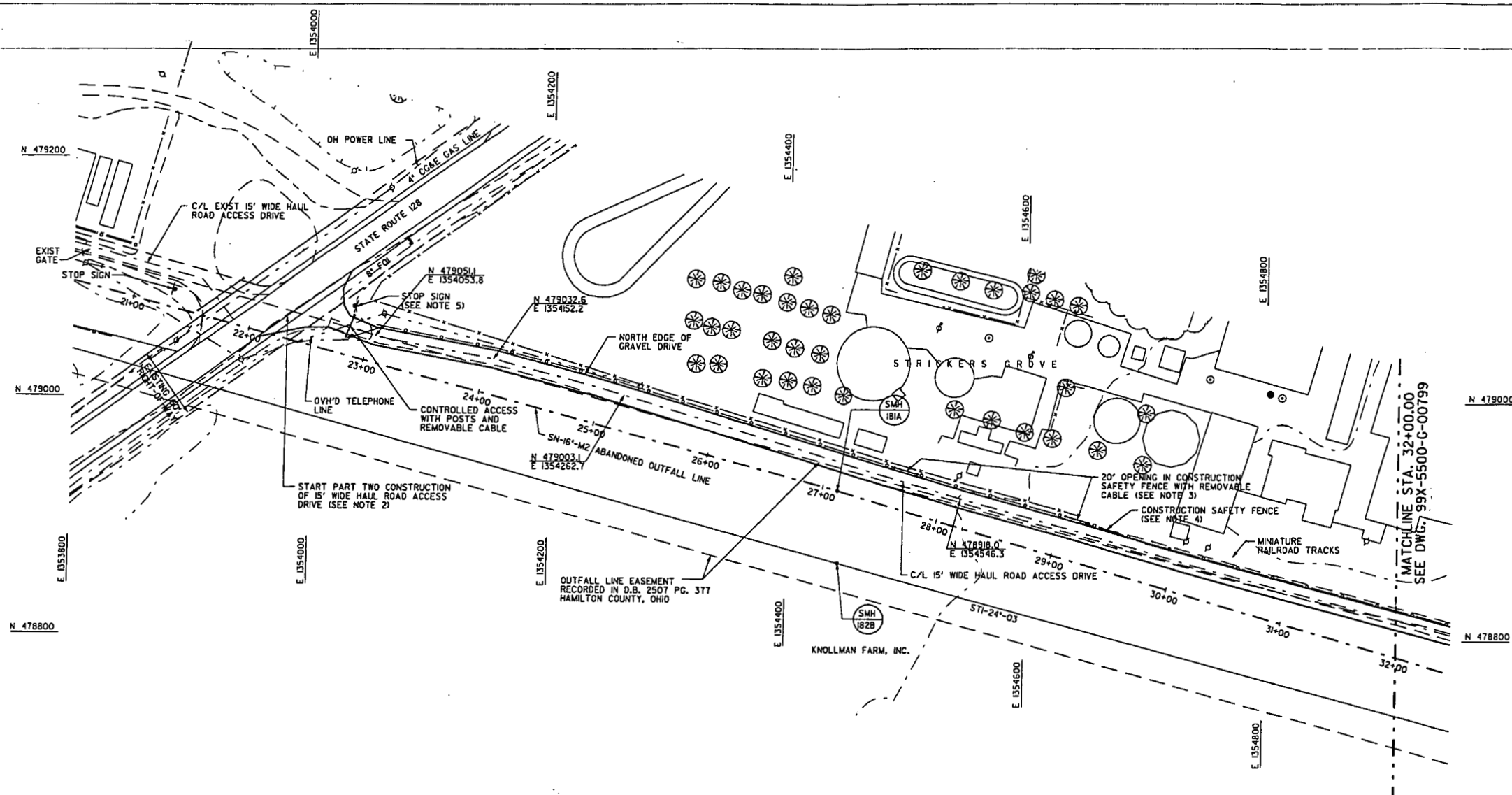
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GENERAL NOTES

1. SEE DRAWING 99X-5500-G-00775 FOR GENERAL AND CONSTRUCTION NOTES.
2. SEE DRAWING 99X-5500-G-00797 FOR HAUL ROAD CROSS SECTION, ROUTE AND DETAILS.
3. LOCATION OF OPENING IN CONSTRUCTION SAFETY FENCE TO BE SELECTED BY THE ADJACENT LANDOWNER.
4. CONSTRUCTION SAFETY FENCE SHALL BE LOCATED A MINIMUM OF 4' FROM EDGE OF MINIATURE RAILROAD TRACK RAIL AND AS CLOSE TO EDGE OF HAUL ROAD ACCESS DRIVE AS POSSIBLE.
5. INSTALL STOP SIGN IN ACCORDANCE WITH REQUIREMENTS OF OHIO DEPARTMENT OF TRANSPORTATION.

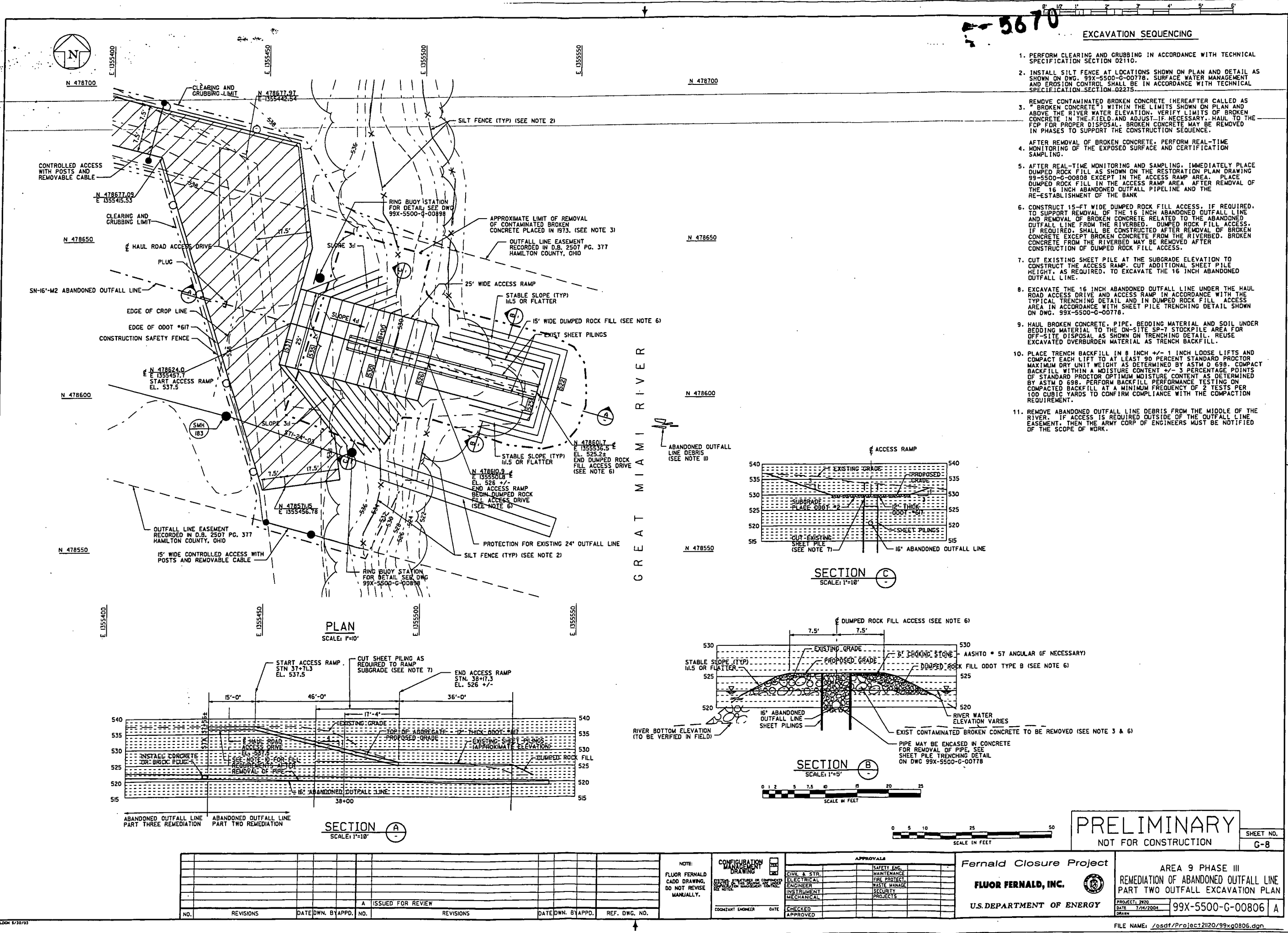


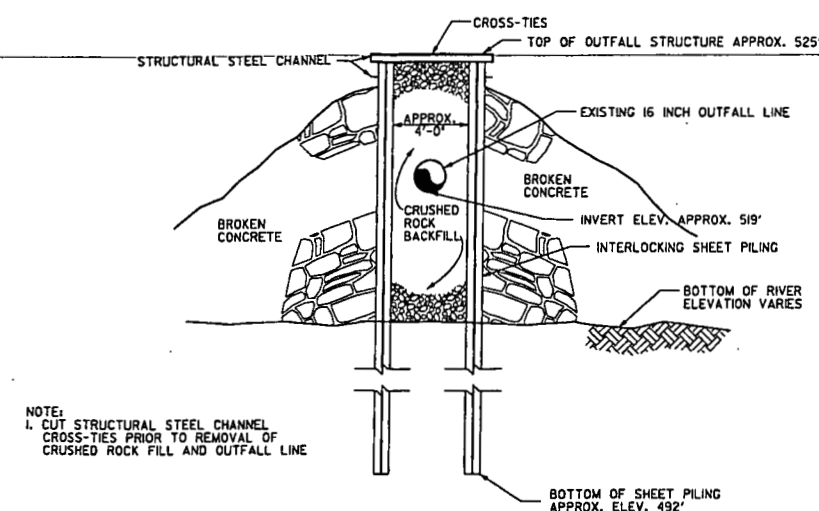
PRELIMINARY
NOT FOR CONSTRUCTION

SHEET NO.
G-6

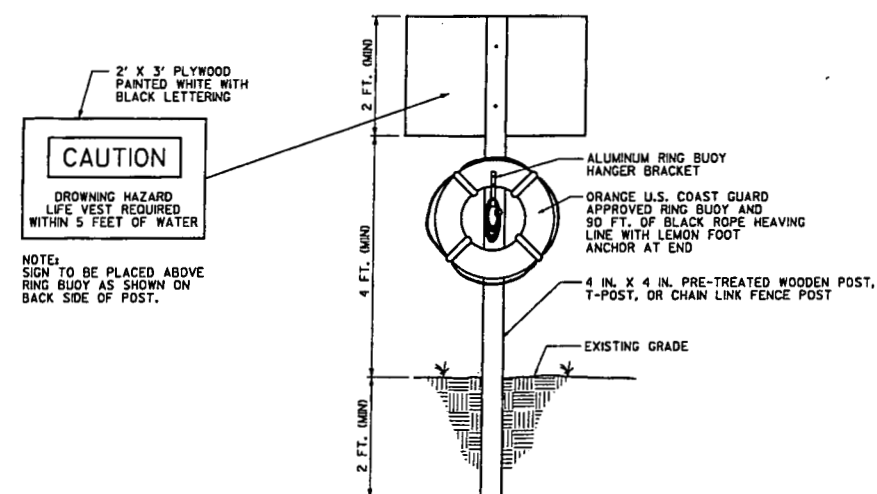
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TYPICAL SHEETPILE TRENCHING DETAIL
NOT TO SCALE



TYPICAL RING BUOY STATION DETAIL
NOT TO SCALE

PRELIMINARY
NOT FOR CONSTRUCTION

[illegible]

NOTE:
FLUOR FERNAL
CADO DRAWING
DO NOT REVISE
MANUALLY.

**CONFIGURATION
MANAGEMENT
DRAWING**

SYSTEMS STRUCTURES OR COMPONENTS
DETAILED ON THIS DRAWING ARE UNDER
CONFIGURATION MANAGEMENT CONTROL.
SEE NOTES.


C. VAN ARSDALE 6/7/91
COORDINATING ENGINEER 047

APPROVALS					
CIVIL & STR.	L. SHOF	7/10/81	SAFETY ENG.	C. JOHNS	1/
ELECTRICAL			MAINTENANCE		
ENGINEER			FIRE PROTECT.		
INSTRUMENT			WASTE MANAGE		
MECHANICAL			SECURITY		
WAG			PROJECTS		
	L. BARR	7/10/81	CONSTRUCTION	B. JOHNS	0
CHECKED	B. LINDOM	6/10/81	CHARACTERIZATION	F. MILLER	1/
APPROVED	SE. RAH	6/10/81	QUALITY CONTROL	N. SCHWY	1/

Fernald Closure Project

FLUOR FERNALD, INC.

U.S. DEPARTMENT OF ENERGY



AREA 9 PHASE III
REMEDATION OF ABANDONED OUTFALL LINE
PART TWO DETAILS

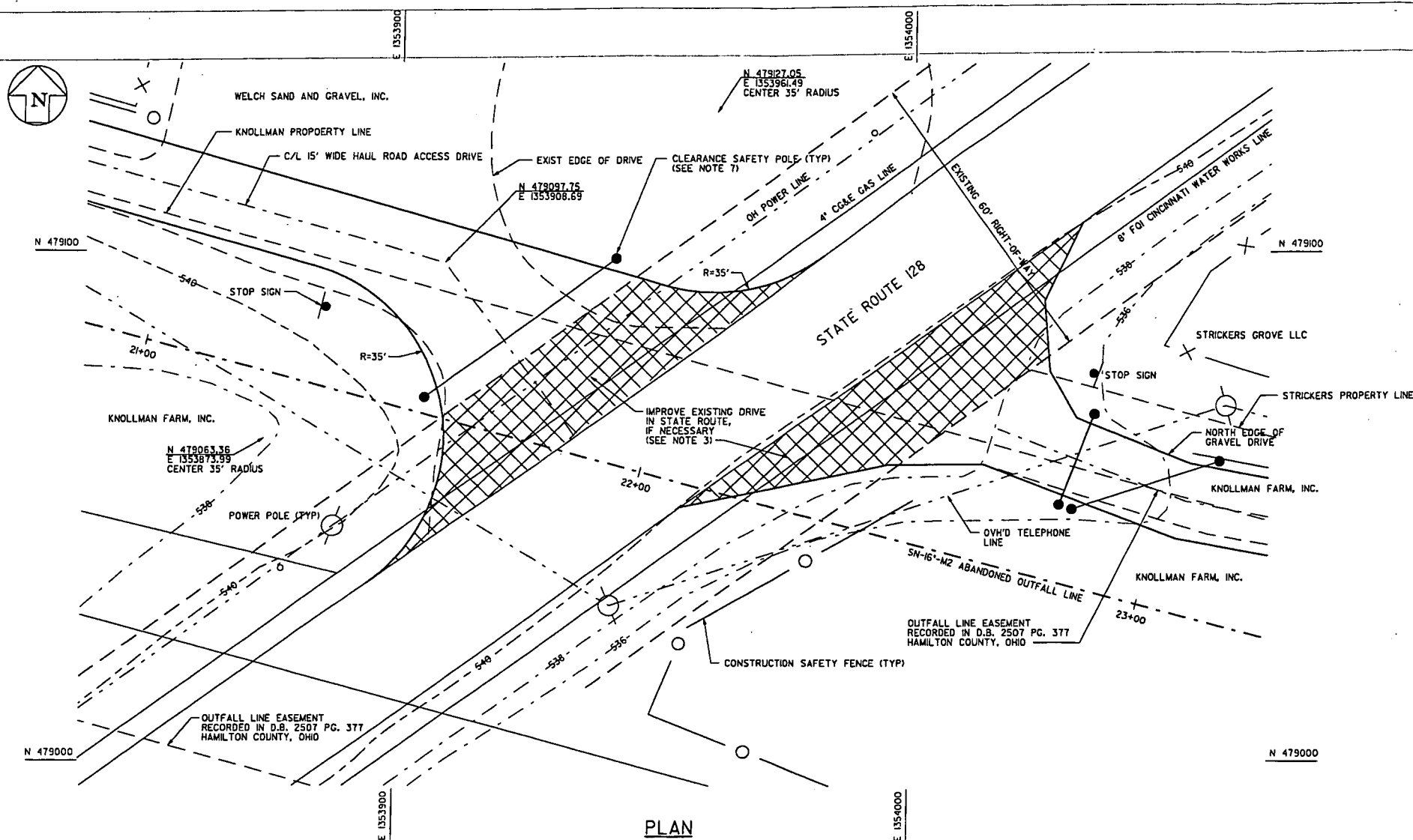
PROJECT: 2820	99X-5500-G-00809	A
DATE: 8/09/2004		

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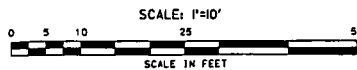
GENERAL NOTES

1. SEE DRAWING 99X-5500-G-00775 FOR GENERAL AND CONSTRUCTION NOTES.
2. SEE DRAWING 99X-5500-G-00778 FOR WORKING AREA AND TRENCHING DETAILS.
3. SEE DRAWING 99X-5500-G-00797 FOR HAUL ROAD CROSS SECTION, ROUTE AND DETAILS.
4. DO NOT OPERATE TRACK EQUIPMENT ON STATE ROUTE. CROSS STATE ROUTE WITH TRACK EQUIPMENT AFTER ROAD PROTECTION MEASURES (PLYWOOD, RUBBER MATTING, ETC.) ARE PLACED OVER THE ROAD.
5. INSTALL TRAFFIC SIGNS "ROAD WORK AHEAD" AND "TRUCK CROSSING" ALONG STATE ROUTE 128 BOTH DIRECTIONS AT 1000 FEET AND 500 FEET RESPECTIVELY FROM THE HAUL ROAD CROSSING. COVER OR TAKE DOWN SIGNS NIGHTLY AFTER EACH DAYS WORK.
6. EQUIPMENT WHICH MAY CROSS STATE ROUTE 128 INCLUDE:
 - 30 TON ARTICULATING TRUCK
 - ROLL-OFF BOX TRUCK
 - CAT 330 TRACK HOE OR EQUAL
 - CAT D6 BULLDOZER
 - CAT 125 LOADER
 - 2000-3000 GALLON WATER TRUCK
7. INSTALL SAFETY POLES WITH WIRE PLACED ACROSS POLES AT A MINIMUM CLEARANCE HEIGHT OF 16.30 FEET. INSTALL STRINGERS ON WIRE.



PLAN

PRELIMINARY
NOT FOR CONSTRUCTION



SHEET NO.
G-II

NO.	REVISIONS	DATE	OWN. BY	APPD. NO.	REVISIONS	DATE	OWN. BY	APPD. NO.	REF. DWG. NO.

NOTE:
FLUOR FERNALD
CADD DRAWING.
DO NOT REVISE
MANUALLY.

CONFIGURATION
MANAGEMENT
DRAWING

DATE: 8/25/2004
DRAWN: R.M. LINDGREN
CHECKED: []
APPROVED: []

APPROVALS	DATE
CIVIL & STR.	
ELECTRICAL	
ENGINEER	
INSTRUMENT	
MECHANICAL	
SAFETY ENG.	
MAINTENANCE	
FIRE PROTECT.	
WASTE MANAGE.	
SECURITY	
PROJECTS	
CONSTRUCTION	
CHARACTERIZATION	

Fernald Closure Project
FLUOR FERNALD, INC.
U.S. DEPARTMENT OF ENERGY

AREA 9 PHASE III
REMEDIATION OF ABANDONED OUTFALL LINE
ST. RT. 128 ROAD CROSSING TRAFFIC PLAN
PROJECT: 2920
DATE: 8/25/2004
DRAWN: R.M. LINDGREN
99X-5500-G-00802 A

SECTION 02150
TRAFFIC CONTROL

5670

PART 1 GENERAL

1.1 SCOPE

- A. This Section includes the traffic control requirements for the excavation areas, Special Materials Transfer Area (SMTA), Stockpile 7 (SP-7) and movement between the following: On-Site Disposal Facility (OSDF), OSDF Material Transfer Areas (OMTA), Bulk Material Transfer Area (BMTA), OSDF borrow area, Waste Pits Remedial Action Project (WPRAP), and designated project operational limits. (Requirements for traffic control within the OSDF and OSDF borrow area are contained in the IMPP and the OSDF Borrow Area Management and Restoration Work Plan.)

1.2 RELATED SECTIONS AND DOCUMENTS

- A. Section 02200 – Earthwork.
- B. Section 02205 – Impacted Material Excavation.
- C. Section 02206 – Earthwork for Remediation.

1.3 REFERENCES

- A. Manual of Uniform Traffic Control Devices (MUTCD) for Streets and Highways, current edition.
- B. State of Ohio, Department of Transportation (ODOT): Construction and Material Specifications, current edition.

PART 2 PRODUCTS

2.1 MATERIALS

- A. Provide materials for traffic control, including stop and yield signs conforming to MUTCD and ODOT specifications.
- B. Provide fencing as specified in Section 02200.

PART 3 EXECUTION

3.1 TRAFFIC CONTROL

- A. Supply, install, and maintain traffic control devices.
- B. Maintain speed limit of construction vehicles and equipment per postings and specific requirements stated in the Construction Traveler Package.

- C. Ensure that haul equipment or other equipment traveling between certified and non-certified areas, remains on roads constructed of certified material. Perform wheel-wash activities and decontamination as necessary. The Construction Manager will arrange for radiological monitoring in accordance with Section 02205.
- D. Provide entry points to the excavation area free of interference from non-project operations. Ensure that traffic entering haul roads yields to traffic already on the road.
- E. Ensure that traffic routes are acceptable for use (stable) daily and after an event (rain) that may have altered the condition of the route.
- F. Routes that cause equipment and/or vehicles to operate on inclines shall be evaluated and operational limits stated.

3.2 IMPACTED MATERIAL HAUL ROAD

- A. Control the Impacted Material Haul Road (IMHR) as a contamination area during excavation and hauling of above-WAC materials. The Construction Manager must approve IMHR crossings from controlled areas.
- B. The Construction Manager may post or reclassify road crossing intersections with the IMHR during periods of inactivity.

3.3 UTILITY CROSSINGS

- A. Protect energized or active utility lines outside of existing paved areas that intersect proposed traffic routes. Use a 1 inch thick steel plate or an equivalent alternative. Provide length and width of steel plates as required to protect the existing utilities.

3.4 EQUIPMENT PARKING

- A. OSDF Contractor Administration Area
 - 1. Provide non-contaminated equipment parking areas within the OSDF Contractor Administration Area.
 - 2. Restrict personal vehicles from the equipment parking areas.
 - 3. Provide personnel parking at the OSDF Contractor Administration Area.
- B. Contaminated Equipment Parking Area
 - 1. Locate the contaminated equipment parking area as close as possible to the Radiological Control Point Facility.
 - 2. Maintain contaminated equipment parking areas free of mud, debris and standing water.
 - 3. Park contaminated equipment utilized in the SP-7 area, or above-WAC areas at the boundary of the area, in accordance with Section 02205, until decontaminated and released from the area.

5-5670

- C. Parking areas shall be constructed to insure that personnel accessing and servicing the vehicles shall have firm and stable footing. The use of rip rap and other like materials is not acceptable.

END OF SECTION

SECTION 02205
IMPACTED MATERIAL EXCAVATION

5670

PART 1 GENERAL

1.1 SCOPE

- A. This Section includes the requirements for excavating, size-reducing, segregating, stockpiling, loading, hauling, and unloading impacted material. Activities include, but are not limited to, the following:
1. Excavation of impacted materials to the design grade.
 2. Segregation of impacted materials for disposition or reuse.
 3. Size reduction of remaining structures, utilities, and miscellaneous debris.
 4. Loading, hauling and unloading of impacted materials to appropriate disposition.
 5. Development and maintenance of project stockpiles.
 6. Support of site monitoring and sampling activities.
 7. Performance of area management activities.
 8. Supplemental excavation beyond the design grade.
 9. Excavation of utilities below the design grade.

1.2 RELATED SECTIONS AND DOCUMENTS

- A. Section 02100 – Surveying.
- B. Section 02150 – Traffic Control.
- C. Section 02200 – Earthwork.
- D. Section 02206 – Earthwork for Remediation.
- E. Section 02230 – Road Construction.
- F. Section 02275 – Surface Water Management and Erosion Control for Remediation.
- G. Section 02930 – Vegetation.
- H. Impacted Materials Placement Plan (IMPP), On Site Disposal Facility (OSDF), 20100-PL-007, current revision.

1.3 REFERENCES

- A. Title 29, Code of Federal Regulations (CFR): 29 CFR 1926 Subpart P – Excavation, current edition.
- B. Waste Acceptance Criteria (WAC) Attainment Plan for On Site Disposal Facility (OSDF), 20100-PL-0014, current revision.

0530 C. RM-0047, Fugitive Dust Control Requirements, current revision.

1.4 DEFINITIONS

- A. Active Stockpile: A stockpile specifically designed to operate for longer than 45 calendar days. This includes interim stockpiles that are older than 45 days and existing stockpiles shown on the Construction Drawings.
- B. Debris: Impacted material that is generated during the excavation and removal of building structures, utilities, miscellaneous man-made materials, and natural materials. These materials include floor slabs; foundation walls; sheet piling; foundations; piers; footings; hydraulic ram casings; structural steel, re-bar and miscellaneous metal; electrical duct-bank, manholes, electrical wiring, and power poles; remaining equipment and miscellaneous mechanical items; chain link and other fencing; agricultural drain tiles; and rock, asphaltic pavement, and other aggregate materials not defined as Special Materials.
- C. Design Grade: Grade created by excavation of impacted material to the lines and grades shown on Construction Drawings.
- D. Final Remediation Levels (FRLs): The permissible concentration of contaminants that may remain in site soil and sediment following completion of remedial actions.
- E. Hazardous Waste Management Unit (HWMU): Area shown on the Construction Drawings that has contained hazardous or mixed waste.
- F. Impacted Material: Soil with contamination levels above the established FRLs or man-made materials. Impacted materials associated with soil excavation projects are presented in Table 02205-1.
- G. Interim Stockpile: An impacted material project stockpile that is intended to be dispositioned in less than 45 calendar days.
- H. Process Piping: Piping that is more likely to contain contaminated residue based on process knowledge of previous excavation activities (i.e. sanitary, effluent, or sump liquor lines).
- I. Real-time Monitoring: Consists of several alternative methods of utilizing in-situ gamma spectroscopy, to analyze contaminant levels on the excavation surface.
- J. Special Materials: Impacted material requiring special handling as specified in this Section and presented in Table 02205-1.
- K. Supplemental Excavation: Removal of impacted material encountered beyond design grade.
- L. Underground Storage Tank (UST): Tank that was used to contain an accumulation of a regulated substance, of which the volume was 10 percent or more beneath the ground surface.
- M. WAC: Waste acceptance criteria (WAC) for disposition of material at the OSDF as defined by the WAC Attainment Plan. This WAC includes radiological/chemical criteria for soil,

physical criteria for debris, and criteria for ancillary remediation waste (i.e., analytical sample returns, PPE).

1.5 SUBMITTALS

- A. Maintain a daily record of underground utilities that have been excavated from below the design grade, in the form of a redline drawing mark-up, for review by the Project Engineer every 30 days following the start of excavation beyond the design grade.
- B. Submit calculations certified by a registered Professional Engineer ensuring slope stability when using equipment having a gross weight greater than specified for a Caterpillar CAT 350L within the excavation area, as necessary.

1.6 VERIFICATION OF THE EXISTING CONDITIONS

- A. Verify existing conditions as specified in Section 02100.

PART 2 PRODUCTS

2.1 MATERIALS

- A. Provide 20-mil sheets of Herculite or equal for use in buffer areas.
- B. Provide 3/8-inch yellow nylon rope fence. Posts shall be in accordance with Section 02200.

2.2 EQUIPMENT

- A. Furnish and maintain equipment to perform required operations in accordance with this Section.
- B. Equipment used to haul impacted material over the existing paved IMHR, shall be equal to or less than the gross vehicle weight and axle loading for a Caterpillar CAT D300E haul truck (gross vehicle weight of 120,000 pounds and maximum axle width of 9-feet 10-inches). Pavement width of the existing two-way IMHR is 24-feet. Select equipment and equipment width to ensure safe operation on this road.
- C. Equipment used within the excavation area shall be equal or less than the gross weight for a Caterpillar CAT 350L track hoe (112,500 pounds). Heavier equipment may be approved for use in this area pending submittal and approval of calculations certified by a registered Professional Engineer, that ensure slope stability.
- D. Equipment used to haul and unload impacted materials shall have enclosed cabs. Enclosed cab is defined as an equipment cab isolated from outside environment (intact windows, doors, panels and floors surrounding driver with windows and doors shut) which provides a barrier from intrusion of outside airborne particles. Heating, ventilating, or air conditioning units associated with the equipment cab must not provide a direct path for outside air to enter (air conditioner on air recirculate mode) unless the air is first passed through a high efficiency particulate air (HEPA) filter pulled directly from outside the cab.

- E. Furnish placards, placard carriers and 3-inch nominal diameter mounted sealable polyvinyl-chloride (PVC) tubes to serve as manifest carriers on haul trucks. Install at locations as directed by the Construction Manager.
- F. Assign equipment used to haul material to the OSDF with unique alpha-numeric identifiers. This identifier shall be placed on both sides of the truck cab and shall be visible from 20 feet.
- G. Equip trucks used for hauling impacted material with automatic load cover tops or employ Best Available Technology (BAT) methods to ensure that no airborne materials are emitted from the haul truck bed or load, whether empty or full.
- H. Provide water tank trucks, water wagons, hydroseeders, portable tanks, pressure distributors, piping, sprinklers or other equipment designed to apply water and/or dust suppressant and crusting agent uniformly and in controlled quantities to variable surface widths to provide fugitive dust control.
- I. Provide pressure wash or comparable equipment necessary to clean visible process residue and soil from piping and debris for placement in the OSDF.
- J. Provide portable wash equipment to wash vehicle tires and vehicle exteriors as necessary.
- K. Provide ~~Equipment~~ used for size reducing concrete and asphalt materials for use as temporary aggregate surfaces.

PART 3 EXECUTION

3.1 GENERAL EXCAVATION

- A. Prior to performing excavation activities, satisfy the following requirements:
 - 1. Complete preliminary survey and layout work in accordance with Section 02100.
 - 2. Establish dust control methods in accordance with RM-0047, "Fugitive Dust Control Requirements", ~~site procedures~~.
 - 3. Install and manage surface water management and erosion and sediment control measures in accordance with Section 02275.
 - 4. Install, modify, and manage construction safety and radiological-control fence and signage. Install posts at spacing recommended by the manufacturer's installation procedures and as required to prevent sagging. Posts with less than 4 feet remaining above the ground after installation shall have safety caps installed.
 - 5. Obtain survey and red line markup of area isolation trench, where applicable, to verify completeness.
- B. Perform excavation activities in compliance with 29 CFR Part 1926.650 through 1926.652.
- C. Unless otherwise noted on the Construction Drawings, the following slope stability requirements shall apply during excavation activities:
 - 1. Excavation slopes with depths less than 20-feet shall be performed in accordance

2. with applicable Occupational Safety and Health Administration (OSHA) guidelines. Sloped excavations greater than 20 feet in depth shall be designed by a Professional Engineer (PE) registered in the state of Ohio.
 3. Temporary excavation slopes with depths less than 20-feet shall be no steeper than 1.5H:1V with a maximum height of 13 feet between 15 foot benches.
 4. Design slopes and slopes created by supplemental excavation shall be no steeper than 2H:1V with a maximum height of 13 feet between 15 foot benches.
- D. Install and manage traffic control devices in accordance with Section 02150.
- E. Blasting, including use of explosives or explosive devices, is not permitted.
- F. As necessary, establish and manage Special Materials Transfer Area(s) (SMTA) as follows:
1. Establish the SMTA on an existing building slab, paved parking area, or new aggregate surface adjacent to the project boundary, accessible from inside and outside the excavation area.
 2. Construct ingress/egress to the SMTA, including access roads, ramps, and drainage improvements as required.
 3. Install new aggregate surface SMTA in accordance with the Construction Drawings and Section 02230, and provide positive drainage.
 4. The SMTA shall not be used as a laydown area.
 5. The SMTA shall be controlled as a radiological buffer area in accordance with this Section.
- G. Establish controls for removal of ACM in accordance with the approved Construction Traveler Package.
- H. Excavate surface material within the limits of excavation to a minimum depth of 2 feet, unless otherwise noted on the Construction Drawings.
- I. Excavate fractured materials concurrent with the adjacent above-WAC or below-WAC material, minimizing generation of above-WAC material. Fracturing of at-grade concrete slabs may continue concurrently with above-WAC and/or RCRA hazardous soil removal, but concrete outside of above-WAC and RCRA hazardous areas shall not be removed until above-WAC and RCRA hazardous soil removal has been completed, unless otherwise approved by the Construction Manager.
- J. Excavate and load material in such a manner that enables visual observation of excavation and loading operations as required to accurately manifest material for disposition.
- K. Excavations shall generally proceed in an up-gradient to down-gradient pattern to the lines and grades shown on the Construction Drawings.
- L. Keep excavation equipment in contaminated areas to prevent recontamination of areas excavated to final grade. Excavate an area by methods that prevent drainage of surface water into the area.
- M. Unless otherwise noted on the Construction Drawings or directed by the Construction

Manager, excavate material from known above-WAC and RCRA hazardous areas in accordance with this Section, prior to excavating below-WAC material

~~M. Unless otherwise noted on the Construction Drawings or directed by the Construction Manager, excavate material from known above-WAC and RCRA hazardous areas in accordance with this Section, prior to excavating below-WAC material.~~

- N. Survey and stake excavation areas as shown on the Construction Drawings, in accordance with Section 02100. Perform intermediate and final surveys as specified in Section 02100 for measurement and to confirm attainment of the design grade.
- O. Remove material adjacent to structures that extend above the excavation surface, as well as the structures themselves, in a safe manner to ensure that an unstable condition does not exist.
- P. Prior to excavating previously trenched and backfilled isolation trenches, physical sampling is required in locations where sanitary and process lines were cut during trenching. Allow 10 working days for physical sampling and analysis.
- Q. Design contours and grades shown on the Construction Drawings represent the minimum limits of excavation required to capture contamination and foundations, while maintaining safe slope requirements. Field changes shall be reported to the Construction Manager and approved by the Project Engineer.
- R. In the event a historic, prehistoric, or archeological site, feature, or object is discovered, stop excavation in the area and immediately notify the Construction Manager for evaluation by Cultural Resources.
- S. Continuously observe excavations for Special Materials or change in materials. In the event a Special Material or change in materials is encountered, stop excavation in the area and immediately notify the Construction Manager for evaluation by WAO. Dispose of Special Materials in accordance with Table 02205-1.
- T. In the event solvent saturated soils are encountered, stop excavation in the area and immediately notify the Construction Manager for evaluation by WAO. Excavate, load, and haul the material as directed by the Construction Manager.
- U. Prevent damage to adjacent structures, materials, and equipment, including utilities that are to remain, or those installed for performance of this work. Repair damage that occurs due to execution of this scope.

3.2 EXCAVATIONS APPROACHING THE GMA

- A. Stop excavation activities in the immediate area and immediately notify the Construction Manager if sands and gravel are encountered, even if the design grade has not been achieved.

- B. Use caution when excavations encroach on the 5-foot protective cover over the unsaturated sands and gravel of the GMA. Approximate GMA elevations are shown on the Construction Drawings.
- C. Prior to initiating work activities ~~which that will encroach on~~ breach the 5-foot protective cover of the GMA, but not extend within 2-feet of the GMA, proceed as follows:
 - 1. Excavate to achieve design grades shown on the Construction Drawings or to remove Above-FRL material.
 - 2. Allow 24 hours for coordination of the following: examination of the excavation bottom to determine if GMA sediments are present, real-time monitoring and/or physical sampling of the area to be backfilled, and identification of necessary pumping requirements for ponded water.
 - 3. Immediately following monitoring and sampling, initiate backfilling to protect the GMA in accordance with Section 02206.
- D. Prior to initiating work activities which will encroach within 2 feet of or breach the GMA, obtain approval and direction from the Construction Manager to proceed as follows:
 - 1. Excavate to achieve design grades shown on the Construction Drawings or to remove Above-FRL material.
 - 2. Allow for real-time monitoring and physical sampling in the area prior to backfilling.
 - 3. Immediately following monitoring and sampling collection, initiate backfilling to protect the GMA in accordance with Section 02206.

3.3 BUFFER AREA MANAGEMENT

- A. Establish excavation boundary and buffer area controls for above-WAC, RCRA hazardous areas, HWMUs, and USTs as follows:
 - 1. Establish excavation area boundaries at surveyed and staked locations, in accordance with this Section.
 - 2. Install T-posts and rope fence at the excavation boundary.
 - 3. Establish the buffer area adjacent to the excavation area to serve as a controlled loading area between the excavation area and the surrounding radiological controlled area.
 - 4. Grade the buffer area to drain into the excavation.
 - 5. Install T-posts and rope fence around the buffer area.
 - 6. Cover the buffer area with a 20-mil sheet of Herculite, or equivalent.
 - 7. Collect water encountered during excavation and pump it as specified in Section 02275 and the construction drawings.
 - 8. Keep the buffer area clean and free of dirt and mud.
 - 9. Remove spillage before releasing haul equipment from the buffer area.
- B. Dedicate and restrict equipment required to excavate, load, haul and place above-WAC, RCRA hazardous and HWMU material to that specific use until decontamination rinsing has been completed and approved by the Construction Manager. Upon approval by the Construction Manager that no visible material is present on exterior equipment surfaces, or

in haul truck beds, equipment may be used elsewhere.

- C. Unless constructability requires that equipment access above-WAC, RCRA hazardous or HWMU excavation areas, excavate these areas by reaching over the excavation area boundary and dumping directly into haul equipment located in the buffer area. Above-WAC, RCRA hazardous and HWMU excavation boundaries may be adjusted as sampling and/or monitoring demonstrates that constituents of concern have been excavated to levels sufficient to down-post portions of the area.
- D. Load haul equipment in a manner that prevents spillage and accumulation of material on the wheels and exterior of the haul equipment. Remove visible material that accumulates on the truck exterior. Multiple loading areas may be established within the buffer area.
- E. Wash haul equipment within the buffer area as necessary (using low volume, high-pressure washer or approved equivalent) to remove above-WAC, RCRA hazardous or HWMU material from the exterior of equipment.

3.4 ABOVE-WAC EXCAVATIONS

- A. Establish excavation boundary and buffer area controls in accordance with this Section.
- B. Remove debris (concrete, asphalt, and miscellaneous structures) from above-WAC areas and pressure wash the debris as directed by the Construction Manager to remove soil/residue. The Construction Manager will visually monitor washing operations to ensure that debris is free of soil/residue and approved for loading and hauling to the OSDF.
- C. Debris from above-WAC areas not approved for disposal at the OSDF shall be dispositioned at SP-7 Waste Pits Remedial Action Project (WPRAP) in accordance with this Section.
- D. Excavate above-WAC soil/gravel in 3+/-1 foot lifts to the design grade, as shown on the Construction Drawings. Load and haul above-WAC soil/gravel to SP-7 WPRAP in accordance with this Section.
- E. The Construction Manager will arrange for real-time monitoring on excavated side slopes after each lift is removed and on the excavation floor upon achieving the above-WAC contamination limits shown on the Construction Drawings. In above-WAC areas contaminated with Tc-99, as shown on the Construction Drawings, the Construction Manager will arrange for physical sampling at the design depth. Allow 10 working days for sampling and analysis. Do not proceed with further excavation in this area until directed by the Construction Manager.
- F. If initial depth of above-WAC material is below the existing surface, excavate below-WAC material in 2+/-1 foot lifts until above-WAC material has been reached, then proceed in 3+/-1 foot lifts.

- G. Pending results of monitoring or analysis, perform supplemental above-WAC excavation beyond design limits shown on the Construction Drawings or in areas not shown on the Construction Drawings, as directed by the Construction Manager.

3.5 RCRA HAZARDOUS EXCAVATIONS

Excavate RCRA Hazardous areas to the limits shown on the Drawings as follows:

- A. Establish excavation boundary and buffer area controls in accordance with this Section.
- B. Remove debris (concrete, asphalt, and miscellaneous structures) from RCRA hazardous areas and pressure wash the debris as directed by the Construction Manager to remove any soil/residue. The Construction Manager will visually monitor washing operations to ensure that debris is free of all soil/residue and approved for loading and hauling to the OSDF. The Construction Manager may require additional washing as necessary to satisfy visual inspection.
- C. Debris from RCRA hazardous areas not approved for disposal at the OSDF shall be stockpiled for treatment by others in accordance with this Section.
- D. Excavate RCRA hazardous soil/gravel in 3+/-1 foot lifts to the limits shown on the Construction Drawing. Load and haul material to an appropriate stockpile for treatment by others in accordance with this Section. Do not proceed with further excavation in this area until directed by the Construction Manager.
- E. The Construction Manager will perform physical sampling upon confirmation that all RCRA hazardous material has been excavated. Allow 10 working days for sampling and analysis.
- F. Perform additional RCRA hazardous excavations, as directed by the Construction Manager, if analysis identifies additional RCRA hazardous material beyond the limits shown on the Construction Drawings.

3.6 HWMU EXCAVATIONS

Excavate material from the HWMU to the limits shown on the Construction Drawings as follows:

- A. Establish excavation boundary and buffer area controls in accordance with this Section.
- B. Remove debris (concrete, asphalt, and miscellaneous structures) from the HWMU and haul to the OSDF as a discrete waste stream. If the HWMU is considered an above-WAC area, load and haul the debris to SP-7WPRAP, or containerize it as directed by the Construction Manager.
- C. Excavate HWMU soil/gravel in 3+/-1 foot lifts to the HWMU design grade, as shown on the Construction Drawings. Load and haul HWMU soil/gravel to the OSDF as a discrete waste stream. If the HWMU is considered an above-WAC area, containerize soil/gravel as directed by the Construction Manager.

- D. The Construction Manager will arrange for physical sampling on excavated side slopes after each lift is removed and on the excavation floor upon achieving the HWMU design grade shown on Construction Drawings. Allow 10 working days for sampling and analysis required for closure of the HWMU. Do not proceed with further excavation in the area until directed by the Construction Manager.
- E. Pending analysis of sample results, perform supplemental HWMU excavation beyond the design grade as directed by the Construction Manager.
- F. Upon confirmation from the Construction Manager that the HWMU excavation is complete, remove HWMU boundary fence, load and haul buffer area Herculite as HWMU debris.

3.7 UST EXCAVATIONS

Excavate USTs and areas that previously contained USTs shown on the Construction Drawings, as well as USTs discovered during excavation, as follows:

- A. Excavate to expose the top of the tank (UST) to allow the Construction Manager to assess the tank's condition.
- B. The Construction Manager and Industrial Hygiene will inspect the tank for the presence of liquid or non-soil residue.
- C. If the tank contains liquid or non-soil residue, as determined by the Construction Manager, proceed as follows:
 - 1. Pump remaining tank contents into drums provided by the Construction Manager.
 - 2. Move filled drums from the excavation area to the SMTA.
 - 3. Drums will be removed from the SMTA by others for appropriate disposition.
- D. Upon confirmation from the Construction Manager that the tank is empty, proceed with removing the tank. Ensure that water does not accumulate in the tank from the surrounding excavation.
- E. During excavation, stage the tank, soil excavated from around the UST, and soil excavated from areas that previously contained USTs separate from other excavated materials.
- F. Upon confirmation from the Construction Manager that the tank is free of visible process residue, size-reduce the tank to meet physical WAC and dispose of it in the OSDF as a discrete waste stream.
- G. If the tank cannot be cleaned of visible process residue, size-reduce the tank for disposition in accordance with this Section.
- H. Over-excavate soil surrounding UST excavations and from areas that previously contained USTs to remove visible stains. Transport this soil as directed by the Construction Manager.

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3.8 BELOW-WAC EXCAVATIONS

Excavate below-WAC material to the limits shown on the Construction Drawings as follows:

- A. Remove slabs, concrete pads, asphalt, gravel, base and sub-base to sub-grade soil within below-WAC areas. Load and haul this material to the OMTA-Bulk Debris or OSDF. The Construction Manager will arrange for real-time monitoring prior to subsequent excavation in the below-WAC area.
- B. Excavate below-WAC areas to the design grade shown on Construction Drawings. Load and haul this material to the OSDF in accordance with this Section.
- C. In below-WAC excavations driven by contamination rather than removal of underground structures, as shown on Construction Drawings, excavate in 3+/-1 foot lifts to the design grade shown on Construction Drawings. The Construction Manager will arrange for real-time monitoring upon removal of each lift. Do not proceed with further excavation in these areas until directed by the Construction Manager.
- D. Over-excavate a minimum of 6 inches in areas where impacted material was stockpiled or pushed for load-out during excavation to allow for visual inspection and disposition of debris that may have been tracked into the soil.
- E. Coordinate real-time monitoring as necessary to minimize delays.
- F. Upon reaching the design grade, notify the Construction Manager for real-time monitoring.

3.9 UTILITY REMOVAL BELOW THE DESIGN GRADE

- A. Excavate known utilities as indicated on the Construction Drawings. Prior to removal, cap, drain, purge and/or plug utility lines to be excavated to prevent release of material into surrounding soil. If fluids or hold-up material is encountered in utility lines, stop work and notify the Construction Manager. Plug sanitary and process lines when liquid flow is detected. Material released from sanitary or process lines will be excavated and dispositioned as directed by the Construction Manager.
- B. If unidentified utilities or underground structures are encountered, notify the Construction Manager, and proceed in accordance with the ~~approved~~ applicable Penetration Permit.
- C. Process piping that is deformed, closed or otherwise hinders visual inspection shall be managed as above-WAC debris based on it's area of origin.
- D. If a utility is suspected of containing ACM, notify the Construction Manager and Industrial Hygiene and manage in accordance with the Construction Traveler Package.
- E. Excavate, size-reduce, and handle piping and debris in such a manner so as to minimize the generation of above-WAC debris or friable asbestos.

- F. Prior to excavating utilities below the design grade, satisfy the following conditions:
 - 1. Complete the intermediate survey, as specified in Section 02100.
 - 2. Obtain Construction Manager approval that real-time monitoring of the design grade is complete.
- G. Excavate and remove utilities in accordance with this Section and details shown on Construction Drawings.
- H. Backfill utilities located below the design grade in accordance with the details shown on Construction Drawings and Section 02206.
- I. If visual monitoring identifies Special Materials during excavation of utilities below the design grade, perform supplemental excavation in accordance with this Section.
- J. Excavate miscellaneous debris encountered below the design grade and disposition in accordance with this Section.
- K. Maintain a daily record of underground utilities excavated from below the design grade.

3.10 MATERIAL SEGREGATION

- A. During excavation, segregate materials by the impacted material categories as defined in the IMPP. Maximize the volume of Category 1 material.
- B. Segregate below-WAC material to support construction of the four zones of each OSDF cell (protective layer, select impacted material layer, impacted material layer, and contouring layer).
- C. Segregate existing surface aggregate material and size-reduced concrete and asphalt materials for use as temporary aggregate material.

3.11 SIZE REDUCTION

- A. Size-reduce remaining structures (i.e., building foundations, slabs, sumps, hydraulic ram casings) located above the design grade to meet OSDF physical WAC.
- B. Size-reduce concrete and asphalt structures (i.e., building slabs, concrete/asphalt pads, roads, parking areas) to maximize the generation of material for temporary aggregate surfaces.
- C. Size-reduce piping and debris to meet OSDF physical WAC in accordance with the IMPP, or size requirements for SP-7 WPRAP in accordance with this Section.
- D. Size-reduce metal materials (i.e., structural steel, piping, equipment, re-bar, miscellaneous metal) in accordance with the IMPP. Load metal components in bulk and haul to the OSDF or OMTA.

- E. Size-reduction shall be by mechanical means, not by flame or torch cutting unless otherwise approved by the Construction Manager.

3.12 GENERAL LOADING AND HAULING

- A. Use paved haul roads designated on the Construction Drawings for hauling where practical or as directed by the Construction Manager. Upon entering the paved haul roads with haul equipment, do not exit except to the excavation area, SP-7WPRAP, OMTA Bulk Debris Area, and/or the OSDF, without approval by the Construction Manager.
- B. Maintain equipment within the excavation area during periods of non-work (evenings, weekends, and holidays) unless equipment decontamination has been completed. Complete decontamination activities and request a radiological survey of the equipment prior to moving equipment out of radiological contamination areas.
- C. When hauling materials to the SMTA, enter the SMTA from the excavation area only.
- D. Load haul equipment in a fashion to minimize load shifting and to prevent spillage during transit.
- E. Extend automatic covers on suitably equipped haul equipment, whether full or empty, during equipment movement.
- F. Keep equipment cab closed and stay within the equipment cab when inside posted contamination area without appropriate PPE except in emergency situations.
- ~~G. Provide material tracking information in accordance with the Material Tracking Plan shown on Construction Drawings.~~
- ~~H.G.~~ Prior to loading and hauling, material designated for the OSDF shall be void of free liquid.
- ~~I.H.~~ Prohibit tracked equipment from hauling, operating, or tracking over the Impacted Material Haul Road (IMHR) or other paved roadways, unless otherwise approved by the Construction Manager.

3.13 GENERAL DISPOSAL

- A. Dispose of impacted material in accordance with this Section. Table 02205-1 provides guidance for the disposition of materials that may be encountered which will be disposed at the three destinations identified in this Section. Notify the Construction Manager in the event that materials are discovered that do not appear to be represented in Table 02205-1.
- B. Pending coordination between the projects, material designated for SP-7WPRAP disposition may be hauled directly to WPRAP. Material hauled directly to WPRAP must comply with SP-7WPRAP disposition requirements presented in this Section.

- C. OSDF disposition requires the following:
1. Compliance with requirements stated in the WAC Attainment Plan and the IMPP.
 2. Piping and debris shall be cleaned of process-related residue in accordance with this Section. Notify the Construction Manager of residue that is not readily removed and manage the debris as directed.
 3. Compliance with criteria identified in Table 02205-1 for management of Special Materials for disposition at the OSDF.
 4. ACM shall be removed, packaged, loaded and hauled in accordance with the Construction Traveler Package.
- D. SP-7WPRAP disposition requires the following:
1. Soil, piping and debris exceeds the requirements for OSDF disposition as outlined in Table 02205-1.
 2. Piping and debris shall be less than 10 inches in at least one dimension, and no longer than 6 feet in any dimension.
- E. SMTA disposition requires the following:
1. Soil, piping and debris exceeds the requirements for OSDF and SP-7WPRAP disposition as outlined in Table 02205-1.
 2. Materials shall be containerized as directed by the Construction Manager.
 3. Loaded containers shall be placed in the SMTA in a manner that protects the containers from damage. Do not stack containers.

3.14 STOCKPILING

- A. Install construction safety fence around active stockpiles in accordance with Section 02200.
- B. Install appropriate signage around the boundary of active stockpiles as directed by the Construction Manager.
- C. Install erosion control measures around active stockpile in accordance with Section 02275.
- D. Maintain fencing, signage, and erosion control measures for the duration of the active stockpile's existence.
- E. Repair damage to active stockpile support structures (i.e., silt fence, perimeter fence) inflicted during performance of this project to their original condition within 24 hours of damage discovery.
- F. Apply crusting agent as specified in Section 02930 within 7 calendar days upon completion of the active soil stockpile or if the active soil stockpile is to be inactive for more than 45 calendar days as directed by the Construction Manager.

- G. Compact/seal the surface of the stockpile in use at the close of each work-day to prevent fugitive dust and erosion.
- ~~H.~~ Construct stockpiles with maximum slopes of 3H:1V and a maximum height to base ratio of 0.2.
- ~~I.H.~~ Locate interim stockpiles within the limit of excavation upon approval from the Construction Manager. Remove interim stockpiles within 45 calendar days.
- ~~I.I.~~ Establish a separate stockpile for solvent saturated soils, as encountered. Manage the stockpile material to avoid cross-contamination with adjacent soil, and to control surface water and dust.
- ~~K.J.~~ Stockpile excavated surface aggregate materials and size-reduced concrete and asphalt as necessary for use as temporary aggregate material.
- ~~L.K.~~ Stockpiles shall not be located within 30 feet of an excavation top-of-slope.
- ~~M.L.~~ Above-WAC and RCRA material, and material not identified in Table 02205-1 that cannot go to the OSDF as is, which is stockpiled within a larger active stockpile area shall be fenced or roped individually in accordance with Section 02200.
- ~~N.M.~~ In addition to general stockpiling requirements, the following requirements shall apply to the management of above-WAC stockpiles:
1. Use dedicated equipment for the preparation and management of the stockpile.
 2. Maintain ingress/egress to the stockpile, including access roads, ramps, and drainage features.
 3. Maintain unloading areas to prevent haul equipment tires from coming in contact with stockpile material.
 4. Locate the stockpile as directed by the Construction Manager.
 5. Decontaminate tools and equipment used to place and manage material within the stockpile prior to requesting release. Following decontamination, the Construction Manager will arrange for a radiological survey to release these tools and equipment.
- O. In addition to general and above-WAC stockpiling requirements, the following requirements apply to the management of stockpiled RCRA material awaiting treatment:
1. Install surface water run-on/run-off controls to minimize water within the area in accordance with Section 02275.
 2. Establish methods for collection of water from within the area.
 3. Install chain link fence immediately around the stockpile to control access.
- P. Cover or lock down soil or debris containing non-friable ACM at the end of the day in accordance with the Construction Traveler Package.

3.15 MONITORING AND SAMPLING

- A. The Construction Manager will arrange for real-time monitoring of below-WAC areas following removal of surface aggregate, concrete and asphalt slabs, pads, roads and parking

areas, and between each lift in contaminated areas until the design grade has been achieved, including the design grade.

- B. The Construction Manager will arrange for real-time monitoring after each excavation lift for above-WAC materials.
- C. The Construction Manager will arrange for radiological monitoring before equipment is released from a buffer area. Working in rain and/or wet weather will increase scanning time.
- D. The Construction Manager will perform visual monitoring during excavation for Special Materials that are not permitted in the OSDF or that will require additional processing to meet WAC. If Special Materials are found, real-time monitoring may be performed to verify removal and determine appropriate disposition of the material.
- E. The Construction Manager will visually monitor piping and debris to ensure no visible process residue remains, in order to be placed in the OSDF.
- F. The Construction Manager will arrange for real-time monitoring to verify that the excavation area has met the requirements for precertification.
- G. When real-time monitoring or sampling is required, excavate in an alternate location within the excavation area while awaiting the results. Excavate alternate locations a minimum of 50 feet from the area being monitored. Allow up to 2 working days for monitoring after area is ready for monitoring. Extend duration for monitoring at least 1 working day for each day precipitation occurs.
- H. The Construction Manager and regulatory agencies may collect samples from the excavation, haul equipment and the OSDF at any time during the project.
- I. The Construction Manager will arrange for monitoring of water collected in suspect VOC areas for the presence of volatile organic compounds (VOCs). If VOCs are detected, the water will be sampled and analyzed to determine disposition.
- J. The Construction Manager will arrange for magnetometer surveys to verify removal of ferrous debris from areas where former underground utilities and structures were removed.

3.16 PRECERTIFICATION AND SUPPLEMENTAL EXCAVATION

- A. The Construction Manager will arrange for real-time monitoring to pre-certify an area as having attained FRL requirements.
- B. Perform supplemental excavations beyond the design grade if either of the following conditions exist:
 - 1. Real-time monitoring and/or physical sampling identifies material beyond the design grade that does not meet FRL requirements.
 - 2. Utilities or other impacted materials identified beyond the design grade.

- C. Install rope fencing and appropriate signage around the pre-certified area perimeter after precertification has been achieved, as directed by the Construction Manager.

3.17 AREA MANAGEMENT

- A. Maintain construction safety fence, radiological-control fence, and stockpile fence as specified in this Section, Section 02200, and as shown on the Construction Drawings.
- B. Management of excavation water shall be as specified in Section 02275.
- C. Perform stabilization of the excavated areas using crusting agent and temporary seeding in accordance with Sections 02275 and 02930.
- D. Notify the Construction Manager prior to removing sediment and debris from ditches, drains and erosion control devices. The Construction Manager will arrange for sampling and analysis of sediment for WAC compliance. Remove and de-water sediment in accordance with the following:
 - 1. Notify the Construction Manager two (2) days prior to removing sediment from ditches, drains and erosion and sediment control devices to determine whether or not sampling and analysis of the sediment is required for disposition.
 - 2. Remove accumulated sediment from ditches, drains and erosion and sediment control devices as directed by the Construction Manager. In no case shall sediment reduce the available depth of the associated feature to less than two-thirds the depth shown on Construction Drawings.
 - 3. De-water sediment removed from above-WAC or RCRA hazardous areas within the associated above-WAC or RCRA hazardous area.
 - 4. De-water sediment removed from ditches, drains and erosion and sediment control devices adjacent to and up-gradient of the associated feature as necessary to allow water to drain immediately back into the feature.
 - 5. Disposition de-watered sediment to the OSDF unless otherwise directed by the Construction Manager.
 - 6. Place sediments accumulated in above-WAC stockpile surface water control devices in the associated stockpile.
- E. Implement seasonal closure methods at the end of each construction season and maintain seasonal closure through winter shutdown. Seasonal shutdown requirements include, but are not limited to:
 - 1. Maintain surface water management and erosion and sediment controls.
 - 2. Maintain dust control, as required.
 - 3. Perform equipment decontamination, as required.
 - 4. Remove water from excavation during seasonal closure, when water depth exceeds 3-foot. Pump water out of the excavations in a manner that maintains the integrity of the design slopes, (i.e., no rapid draw down) and prevents an overflow condition. The pumping priority for open excavations is as follows:
 - a. Open excavation areas containing impacted material that are located up-gradient of remediated areas,
 - b. Completed excavations, and

- c. Other uncompleted excavations.
- 5. Remove sediment and debris from sediment control basins and ditches in accordance with this Section.
- 6. Seed/stabilize stockpiles.

END OF SECTION

Table 02205-1

The table should be interpreted to read that material in column 1 may be dispositioned at the appropriate destination in columns 2-4 if the corresponding criteria are met.

MATERIAL	DESTINATION		
	OSDF	SP7/WPRAP ⁽¹⁾	SMTA ⁽²⁾
Soil from general/UST excavation	• Meets WAC	• Above-WAC	• Not applicable
Soil from RCRA hazardous excavation	• Prohibited	• Prohibited	• Containerize
Soil from HWMU excavation	• Meets WAC	• From an Above-WAC area	• RCRA Hazardous
Soil requiring treatment	• Prohibited pending treatment and direction by the Construction Manager	• Prohibited pending treatment and direction by the Construction Manager	• Fails treatment
Debris from general/UST excavation	• Meets WAC	• Above-WAC • Visible residue cannot be removed	• Not applicable
Debris from above-WAC excavation	• Meets WAC and free of visible soil/residue	• Visible soil/residue cannot be removed	• Not applicable
Debris from RCRA hazardous excavation	• Meets WAC and free of visible soil/residue	• Above-WAC and free of visible soil/residue	• Visible soil/residue cannot be removed
Debris from HWMU excavation	• Meets WAC	• Above-WAC and free of visible soil/residue	• Above-WAC and visible soil/residue cannot be removed
Asbestos ⁽³⁾	• Meets WAC and passes visual inspection by CM	• Above-WAC and non-friable • Visible soil/residue cannot be removed and non-friable	• Friable and fails visual inspection by CM
Non-pressurized Containers ⁽³⁾	• Meets WAC, free of visible soil/residue, and contains no free liquid, product, etc	• Above-WAC and contains no free liquid, product, etc. • Visible soil/residue cannot be removed and contains no free liquid, product, etc. • Has been crushed	• Contains free liquid, residue, etc • Has been crushed
Piping/Pumps ⁽³⁾	• Meets WAC, free of visible soil/residue, and contains no free liquid	• Interior not visible and contains no free liquid • Visible soil/residue cannot be removed and contains no free liquid	• Above-WAC/HWMU and visible soil/residue cannot be removed • Above-WAC/RCRA and visible soil/residue cannot be removed
Transformers/Electrical Equipment ⁽³⁾	• Meets WAC and free of visible soil/residue	• Visible soil/residue cannot be removed and drained of all fluid	• Contains fluid • Above-WAC/HWMU and visible soil/residue cannot be removed • Above-WAC/RCRA and visible soil/residue cannot be removed
Brick including Acid Brick ⁽³⁾	• No acid brick	• Acid brick	• Not applicable
Lead Acid Batteries ⁽³⁾	• Prohibited	• Prohibited	• Containerize
Medical Infectious Waste ⁽³⁾			
Non-soil Residue ⁽³⁾			
Pressurized Containers ⁽³⁾			
Tires ⁽³⁾			
Uranium Metal ⁽³⁾			
Sealed radiological calibration sources			

(1) Must meet SP7 (or WPRAP) size requirements per this Section (2) SMTA materials must be containerized. (3) Special Material per this Section.

• These are final destinations beyond which the project bears no responsibility for material movement. The OMTAs are staging areas prior to OSDF disposition.

SECTION 02206
EARTHWORK FOR REMEDIATION

PART 1 GENERAL

1.1 SCOPE

- A. This Section includes requirements for earthwork associated with remediation of the former production area to include, but is not limited to:
1. Excavation of fill material.
 2. Excavation and placement of materials for ditches and berms.
 3. Placement of fill material in trenches excavated for utility removal in areas below design grade.
 4. Placement of plug material over the unsaturated sands and gravels of the Great Miami Aquifer (GMA).
 5. Interim grading for drainage and road preparation.
 6. Final grading.

1.2 RELATED SECTIONS AND DOCUMENTS

- A. Section 02100 – Surveying.
- B. Section 02150 – Traffic Control.
- C. Section 02205 – Impacted Material Excavation.
- D. Section 02230 – Road Construction.
- E. Section 02275 – Surface Water Management and Erosion Control for Remediation.
- F. Section 02930 – Vegetation.

1.3 REFERENCES

- A. American Society for Testing and Materials (ASTM):
1. ASTM C150 Specification for Portland Cement, current edition.
 2. ASTM D698 Test Method for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lb/ft), current edition.
 3. ASTM D2216 Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock, current edition.
 4. ASTM D2487 Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System), current edition.

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|----|------------|--|
| 5. | ASTM D2922 | Standard Test Methods for Density of Soil and Soil-Aggregate in Place by Nuclear Methods (Shallow Depth), current edition. |
| 6. | ASTM D3017 | Standard Test Methods for Water Content of Soil and Rock in Place by Nuclear Methods (Shallow Depth), current edition. |
| 7. | ASTM D4643 | Test Method for Determination of Water (Moisture) Content of Soil by Microwave Oven Method, current edition. |

1.4 DEFINITIONS

- A. Fill Material: Non-impacted soil obtained from within the excavation area, below the design grades, following pre-certification, or from a designated borrow area.
- B. Design Grade: Grade created by excavation of impacted material to the lines and grades shown on Construction Drawings.
- C. GMA Plug: Non-impacted clay material from the certified Borrow Area used to maintain a minimum protective cover thickness over, and seal breaches into, the GMA unsaturated sands and gravels.
- D. Clay Plug: Same material as specified for GMA Plug used to create a protective cover thickness over pilings and/or foundations to be cut off and left in place.

1.5 SUBMITTALS

- A. Submit for approval, name, address, and qualifications of an independent soil testing laboratory and resume(s) of field technician(s).
- B. Within seven (7) calendar days of obtaining samples and performing field tests, provide copies of lab and field tests performed by the soil testing laboratory and Contractor performing field tests. Soil test results shall include Standard Proctor moisture density tests, sieve analysis, density tests, and Proctor curves for each type of material to be used prior to its use. Field test results shall include a map depicting locations and depth/lift.
- C. Submit specification sheet and Material Safety Data Sheet (MSDS) for sodium bentonite grout.
- D. Submit specification sheet and MSDS for Portland cement.
- E. Submit as-built survey to verify completion of design excavation in accordance with Section 02100.
- F. Documentation of nuclear density gauge calibration in accordance with manufacturer's requirements.

1.6 EXISTING CONDITIONS

- A. Verify existing conditions as specified in Section 02100.

PART 2 PRODUCTS

2.1 MATERIALS

- A. Fill material shall be free of debris, foreign objects, large rock fragments (maximum dimension of 6 inches), organic, and other deleterious materials.
- B. GMA plug material shall be non-impacted clay material conforming to CL classification according to Unified Soil Classification System in accordance with ASTM D2487.
- C. Use the designated certified borrow area as the source for GMA plug material.
- D. Material used to backfill utility trenches located below the design grade and areas where grout plugs are installed into air-gapped storm sewers shall be surrounding soils following pre-certification.
- E. Use below-FRL material from the excavation area, following pre-certification, as a source of fill material for supplemental excavations.
- F. Portland cement per ASTM C150, normal – Type 1.
- G. Sodium bentonite grout.
- H. Safety signs around ponds shall be exterior quality signs with minimum 4 inch high letter and shall state, "Life jackets required when working within 5 feet of edge of ponds, use of buddy system required". Hand written signs are unacceptable.

2.2 EQUIPMENT

- A. Furnish and maintain equipment to perform required operations in conformance with the requirements of these specifications.
- B. Furnish equipment to perform required operations in conformance with this Section. Equipment that results in waste or damage of material, inaccurate work, or is otherwise objectionable shall be promptly replaced.
- C. Equipment used to haul non-impacted material over the existing paved Impacted Material Haul Road (IMHR), shall be equal to or less than the gross weight and axle loading for a Caterpillar CAT D300E haul truck (gross vehicle weight of 120,000 pounds and maximum axle width of 9-feet 10-inches). Pavement width of the existing two-way IMHR is 24-feet. Select equipment and equipment width to ensure safe operation on this road.

- D. Equipment used within the excavation area shall be in accordance with Section 02205.
- E. Furnish compaction equipment, as needed.

PART 3 EXECUTION

3.1 GENERAL

- A. Perform intermediate surveys in accordance with Section 02100, to confirm attainment of design grade prior to initiating earthwork activities below the design grade.
- B. Stop excavation activities and immediately notify the Construction Manager upon discovery of unexpected cultural resources suspected to be historic, prehistoric, or archeological site, feature or object.
- C. Excavate soil below the design grade with equipment that has been cleaned off free of visible above-FRL soil from prior above-FRL excavation activities.
- D. Use material from the immediate excavation area or a designated borrow area to meet fill material requirements for trenches and supplemental excavations.
- E. Perform construction activities such that surface water runoff from non-certified construction areas does not flow into pre-certified areas, in accordance with Section 02275.
- F. Maintain slope stability in the area surrounding outside of utility trenches created to excavate by excavating utilities below the design grade, in accordance with Section 02205.
- G. After certification, perform final grading in accordance with this Section.

3.2 BACKFILL

- A. Obtain fill material from areas approved by the Construction Manager. In high-leachability areas shown on the Construction Drawings, obtain fill material from precertified or certified areas meeting the 20 mg/kg total Uranium FRL.
- B. Maintain a minimum 5-feet protective cover over the unsaturated sands and gravel of the GMA in accordance with the following. GMA elevations are shown on the Construction Drawings.
 - 1. Following excavation activities that encroach on the 5-feet protective cover but do not extend within 2-feet of the GMA, install GMA plug as follows:
 - a. Immediately following monitoring and sampling activities specified in Section 02205, survey the excavation and initiate filling.

- b. Backfill using GMA plug material meeting the requirements of this Section. Complete backfilling within 5 days following sampling.
 - c. Backfill GMA plug material in 8-inch +/- 1-inch loose lifts until protective cover is returned to a minimum thickness of 5-feet.
 - d. Compact fill material in each 8-inch loose lift to at least 95% Standard Proctor dry density (ASTM D698) within 0 to +3% of optimum moisture content.
2. Following excavation activities that encroach within 2 feet of or breach the GMA, install GMA plug as follows:
- a. Immediately following monitoring and sampling activities specified in Section 02205, survey the excavation and initiate backfilling.
 - b. Backfill using GMA Plug material meeting the requirements of this Section.
 - c. If precipitation is likely within the next 24 hours, immediately place a minimum fresh compacted thickness of 2 feet. Otherwise, place the 2 feet of compacted cover within 24 hours of excavating to within 2 feet or breaching the GMA.
 - d. Place the first lift of GMA plug material in an approximate 18-inch loose lift. Compact the first lift using 4 passes of compaction equipment approved by the Construction Manager.
 - e. Backfill the remaining lifts in 8-inch +/- 1-inch loose lifts until the protective cover is returned to a minimum of 5 feet. Compact GMA plug material in each 8-inch loose lift to at least 95% Standard Proctor dry density (ASTM D698) within 0 to +3% of optimum moisture content. These lifts must be continuous over the entire breached area.
- D. Install clay plug in locations shown on the Construction Drawings in accordance with the following:
- 1. Backfill using clay plug material meeting the requirements of this Section.
 - 2. Place clay plug material in 8-inch +/- 1-inch loose lifts as necessary to achieve a total compacted thickness of 2-feet.
 - 3. Compact each lift with four (4) passes of compaction equipment approved by the Construction Manager.
 - 4. If clay plug is placed over an open pile, seal void spaces within the pile with sodium bentonite grout and cap the top 2 feet of the pile with Type 1 Portland cement ASTM C150 prior to placing the clay plug. Sodium bentonite grout shall be mixed at a water ratio of 2.1 pounds of sodium bentonite per gallon of water, and have a minimum density of 9.4 lbs./gallon. Grout viscosity shall be field checked periodically to assure proper viscosity of 70 +/- 6 seconds using the Marsh funnel viscometer. Tremie the grout into the open pile to eliminate void space.
- E. Excavate, remove utilities, and backfill trenches below the design grade in accordance with the following:

1. Remove utilities located below the design grade in accordance with Section 02205 and details presented on Construction Drawings.
2. Remove water collected in trenches to sump areas and pump it to the appropriate sediment control basin as specified in Section 02275.
3. Backfill utility trenches located below design grade in accordance with the Construction Drawings.

3.3 FIELD QUALITY CONTROL

- A. In-place density testing will be performed in accordance with ASTM D2922. Nuclear density gauge (ASTM D2922) will be calibrated in accordance with the manufacturer's requirements. Documentation of this calibration will be provided to the Construction Manager. Register nuclear or radiological sources brought on site.
- B. Perform in-place moisture tests in accordance with ASTM D3017, or ASTM D4643 as applicable.
- C. Determine moisture-density curves in accordance with ASTM D698 (Standard Proctor). Test results must be reviewed and approved by the Construction Manager.
- D. If in-place density and/or moisture tests indicate that work does not meet specified requirements, replace or re-compact to specified requirements.
- E. Perform soil classification in accordance with ASTM D2487.
- F. Frequency of Tests: Frequency of in-place density and moisture testing shall be whichever of the following requires the greatest number of test:
 1. Once each day when compacting GMA plug material.
 2. Once each compacted lift of GMA plug material.
 3. Once every 3,000 sq. ft. of compacted GMA plug material.
- G. Notify the Construction Manager of activities requiring testing/inspection a minimum of 24 hours prior to the start of such activities.

3.4 EARTHEN BERMS

- A. Install earthen berms at locations shown on the Construction Drawings using soils from surrounding area following pre-certification.
- B. Place material in 8-inch loose lifts.
- C. Compact each lift of berm with four (4) passes of compaction equipment approved by the Construction Manager.

3.5 INTERIM GRADING

- A. When the design grade has been achieved, perform interim grading as follows:

1. Correct washouts or other similar irregularities to maintain the design grade slopes of 2H:1V or less.
2. Grade to maintain smooth continuous slopes.
3. Finish ditches so they drain readily.
4. Perform temporary seeding in accordance with Section 02930.
5. Repair damage within 3 working days.

3.6 FINAL GRADING

- A. Obtain verification that the area to be graded has been certified.
- B. Provide, maintain and operate temporary drains, ditches, pumps, drainage lines or other equipment to intercept, divert, or remove water from excavations.
- C. Provide and maintain stormwater management measures that assure isolation of stormwater between certified and non-certified areas.
- D. Regrade the certified area design grades to achieve grades no steeper than 5H:1V. As the 5H:1V excavation proceeds, the cut material shall be placed in the bottom of existing excavations and graded level.
- E. Final grading shall be achieved with minimum five (5) foot radii at contour direction changes, and smooth transitions between grade breaks and depressions.
- F. Perform permanent seeding in accordance with Section 02930.
- G. Stabilize 2H:1V slopes along excavation boundary with erosion control blankets in accordance with Section 02275.

3.7 SURFACE WATER MANAGEMENT

- A. Manage surface water in accordance with Section 02275.
- B. Perform excavation in a manner that promotes positive drainage.
- C. Install earthen berms as shown on Construction Drawings. Apply temporary seed and fertilizer on earthen berms in accordance with Section 02930.

3.8 SEASONAL SHUTDOWN

- A. Perform seasonal shutdown activities in accordance with Section 02205.

END OF SECTION

SECTION 02275
SURFACE WATER MANAGEMENT AND EROSION CONTROL FOR REMEDIATION

PART 1 GENERAL

1.1 SCOPE

A. This Section includes, but is not limited to, the following:

1. Installation of erosion and sediment control measures.
2. Maintenance of existing and new erosion and sediment control measures installed under this activity, including removal of temporary erosion control facilities.
3. Management of excavation water in excavation areas, including pumping water to the appropriate sediment basin during general excavation and management of volatile organic compound (VOC)-contaminated water.
4. Modification of existing storm sewer system.
5. Installation and maintenance of runoff/runoff controls along the perimeter of the project boundary.
6. Protection of the unsaturated sands and gravels of the Great Miami Aquifer (GMA) from runoff within the excavation areas, including installation and maintenance of runoff controls.
7. Management of area for precertification.
8. Stabilization of disturbed excavation areas or stockpiles.
9. Inspection requirements.

1.2 RELATED SECTIONS AND DOCUMENTS

- A. Section 02100 – Surveying.
- B. Section 02205 – Impacted Material Excavation.
- C. Section 02206 – Earthwork for Remediation.
- D. Section 02270 – Surface Water Management and Erosion Control
- E. Section 02930 – Vegetation.

1.3 REFERENCES

- A. State of Ohio, Department of Natural Resources (ODNR), Rainwater and Land Development, Ohio's Standard for Stormwater Management Land Development and Urban Stream Protection, current edition.
- B. State of Ohio, Department of Transportation (ODOT), Construction and Material Specifications, current edition.
- C. Surface Water Management Plan (SWMP) for the applicable soil excavation project.

1.4 DEFINITIONS

- A. Excavation Water: The combination of surface water and perched water that collects in the excavation.

1.5 SUBMITTALS

- A. For each product proposed for use, submit the following:
1. Manufacturer's product data and recommended methods of installation and maintenance.
 2. Certification from manufacturer that the product meets the material requirements of this Section, including test results.
 3. Material Safety Data Sheet (MSDS), if applicable.
- B. Records of inspection of erosion and sediment control measures as specified herein shall be submitted monthly upon completion of the inspection report.

PART 2 PRODUCTS

2.1 MATERIALS

- A. Furnish silt fence in accordance with ODNR, composed of, at a minimum, strong rot-proof polymeric fibers formed into a woven fabric having fabric and fence post properties as shown on the Construction Drawings.
- B. Furnish woven yarn blanket-like erosion mat that will resist degradation for a minimum of 6-months after installation, having a permissible velocity of 7 feet per second, with the following material properties:
1. Yarn content: 100 percent jute or coconut fiber.
 2. Weight: minimum 11.5 ounces per square yard.
 3. Open Area: maximum 65 percent.
 4. Mesh Opening: minimum 0.5 inches.
- C. Furnish metal staples specifically made to anchor erosion control blankets. Anchors will be 11 gauge wire formed into a staple shape with minimum dimensions of 6 inches by 1 inch by 6 inches.
- D. Furnish dust suppression/crusting agent in accordance with Section 02930.
- E. Furnish backfill in accordance Section 02206.

2.2 EQUIPMENT

- A. Furnish portable tank (minimum 3,000 gal.), as needed to hold water contaminated with volatile organic compounds (VOCs).

- B. Furnish pumps, filters, hoses and other appurtences required to execute work specified in this Section.

PART 3 EXECUTION

3.1 GENERAL

- A. Construct and maintain erosion and sediment control measures as specified in this Section and shown on Construction Drawings. Maintain existing erosion and sediment control facilities and measures in accordance with this Section.
- B. Minimize runoff into disturbed excavation areas by grading the surrounding area away from the disturbed area and/or by constructing temporary diversions as shown on Construction Drawings.
- C. As the excavation progresses, excavate sumps at resulting low points used for water collection of excavation water. Do not penetrate to within 5 feet of the GMA with sump excavations.
- D. Dewater excavations in accordance with the Construction Drawings, if applicable. Excavation water collected within active excavations and below design grade utility removal shall be pumped to the appropriate sediment control basin as shown on Construction Drawings. Collected water shall be pumped down to a depth of less than one foot remaining in the bottom of the excavation within 3 days of the last rainfall.
- E. The Construction Manager will arrange for sampling and analysis of excavation water present in suspect VOC areas prior to a discharge event. Following sampling and analysis, pump collected VOC-contaminated water to the appropriate treatment and/or disposition.
- F. Plug storm sewers at the excavation boundary prior to removal of storm sewer piping located within the excavation area. Plug storm sewer system in such a manner to minimize water collected in abandoned portions of the system.
- G. Remove erosion and sediment control measures after the disturbed excavation areas are stabilized as specified in Section 02930.

3.2 SILT FENCES

- A. Install silt fence at locations down-gradient of areas to be disturbed until drainage and erosion control structures have been established as shown on the Construction Drawings. Remove and dispose accumulated sediment as specified in Section 02205.
- B. Install breaks and overlaps in silt fence as necessary to allow equipment access to construction areas.

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3.3 EROSION CONTROL BLANKETS

- A. Install and maintain erosion control blankets in accordance with manufacturer's recommendations. Install additional staples as necessary to maintain erosion control blankets taut to the ground surface.

3.4 SEDIMENT BASINS AND DITCHES

- A. Remove accumulated sediment and debris from sediment basins and ditches. In no case shall sediment build up to a depth greater than 18 inches in the sediment basin or to a depth greater than one-half the constructed depth of the ditch.
- B. Dispose of sediment and debris as specified in Section 02205.

3.5 GMA PROTECTION

- A. When possible, limit excavations within 5 feet of the GMA to periods of dry weather.
- B. Construct bench terrace or interceptor ditch directly above the excavation extending within 5 feet of the GMA, observing slope stability requirements for the excavation. Slope bench terrace directly above the GMA toward the excavated slope as shown on the Construction Drawings. Grade the bench terrace or interceptor ditch to a sump for pumping to prevent runoff into the GMA.
- C. Size and locate pumps for sumps above the GMA such that no water within the excavation area shall overflow into the GMA from a 10-yr, 24-hr or lesser storm event. Sump bottoms shall not extend to within 5 feet of the GMA.
- D. For excavations extending to within 5-feet of the GMA elevation shown on the Construction Drawings, maintain less than three foot of standing water in the excavations by pumping excavation water to a sediment basin or an adjacent excavation, as necessary.
- E. Backfill over the GMA in accordance with Section 02206.
- F. In areas where 2 feet of compacted cover has been recently restored following excavation which has extended to within 2 feet of or breached the GMA, pump ponded water from the affected excavation as soon as practical (but within 24 hours or by the direction of the Construction Manager) following a precipitation event.

3.6 PRECERTIFICATION

- A. Pipes that daylight at the excavation face shall be plugged at the excavation face prior to precertification in accordance with Section 02205 and the Construction Drawings.
- B. Install run-on controls along perimeter of precertification boundary, as shown on the Construction Drawings, to prevent surface water from non-certified areas from flowing into precertified areas.

- C. During precertification, direct drainage from precertified areas to the sediment basin. Routing of surface water pump lines must be approved by the Construction Manager.
- D. Maintain surface water management within the area to be precertified in accordance with this Section until precertification is complete.

3.7 INSPECTION

- A. Inspect erosion and sediment control measures in accordance with Section 02270.
- B. File records of inspections in accordance with Section 02270.

END OF SECTION

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OSDF PHIV-SPEC REV 0
Section 02100: Surveying

SPECIFICATION COVER SHEET

SPECIFICATION SECTION: 02100 TITLE: SURVEYINGSpecifications By: Signature
(Cognizant Engineer)Printed Name Michael J. Monteleone, P.E.
and Title Associate17 Aug 01
Date

Scope and Format

Checked By: Signature
(Checker)Printed Name Michael J. Monteleone, P.E.
and Title Associate17 Aug 01
Date

Detailed Requirements

Checked by: Signature
(Checker)Printed Name David K. Phillips
and Title Senior Project Engineer17 Aug 01
DateOverall Review By: Signature
(PDP)Printed Name J.F. Beech, Ph.D., P.E.
and Title Principal20 Aug 2001
DateApproved by: Signature
(DTL)Printed Name J.F. Beech, Ph.D., P.E.
and Title Principal21 Aug 2001
Date

Record of Revision (Number and initial all revisions)

Rev. No.	Reason	Date	By	Checked	Approval
0	Certified for Construction	20 August 01	WJM	DKP	JFB
1	Revisions from Phase III DCNs	13 March 02	DKP	DKP	JFB

SECTION 02100

SURVEYING

PART 1 GENERAL

1.01 SCOPE

- A. This Section includes the requirements for surveying. Requirements include, but shall not be limited to:
 - 1. establishing permanent and temporary survey benchmarks and control points;
 - 2. establishing a horizontal and vertical project control system based on existing benchmarks;
 - 3. setting limits and boundaries of construction activities;
 - 4. performing support surveys and surveys for conformance checks, "red-line" drawings, and sketches, and to determine measurement of quantities for periodic progress payments and final payment;
 - 5. preparing and furnishing "red-line" drawings and sketches; and
 - 6. surveys for Excavation Project.

1.02 RELATED SECTIONS AND PLANS

- A. Section 02110 - Clearing, Grubbing, and Stripping
- B. Section 02200 - Earthwork
- C. Section 02205 - Impacted Material Excavation
- D. Section 02206 - Earthwork for Remediation
- E. Section 02207 - Area Isolation Trenching
- F. Section 02215 - Trenching and Backfilling
- G. Section 02225 - Compacted Clay Liner and Cap
- H. Section 02230 - Road Construction
- I. Section 02240 - Non-Impacted Protective and Contouring Layers
- J. Section 02250 - Vegetative Soil Layer
- K. Section 02270 - Surface-Water Management and Erosion Control

- L. Section 02271 - Riprap
- M. Section 02275 - Surface Water Management and Erosion Control for Remediation
- N. Section 02280 - Biointrusion Barrier
- O. Section 02605 - High-Density Polyethylene (HDPE) Pipes and Fittings
- P. Section 02710 - Granular Drainage Material
- Q. Section 02712 - Granular Filter Material
- R. Section 02721 - Culverts
- S. Section 02770 - Geomembrane Liner and Cap
- T. Section 02831 - Chain-Link Fences and Gates
- U. Section 02920 - Topsoil
- V. Section 13000 - Borrow Area Management
- W. Section 13005 - Liner Penetration Boxes
- X. Section 13010 - Impacted Materials Placement
- Y. Impacted Materials Placement (IMP) Plan
- Z. Part 6 - Statement of Work
- AA. Part 8 - Environmental Health & Safety/Training Requirements
- BB. Part 9 - Quality Assurance Requirements

1.03 REFERENCES

- A. National Geodetic Survey (NGS) Standards.

1.04 QUALIFICATIONS

- A. Oversight for the survey work shall be provided and certified by a Land Surveyor licensed in the State of Ohio.
- B. Surveying work shall be performed under the direct supervision of a person who has at least 5 years of experience in construction surveying.

- C. Work performed in referencing or re-establishment of land or United States survey monuments shall be signed and sealed by a Land Surveyor licensed in the State of Ohio.

1.05 SUBMITTALS

- A. Submit a copy of Land Surveyor's license and a résumé of the person supervising the surveys to the Construction Manager within 10 calendar days from Notice to Proceed.
- B. For each liner and cap submit two copies of proposed control points on a minimum 50-foot grid for verification of the following surfaces at least 3 days prior to commencement of work:
 - 1. subgrade for roads, clay liner, and for other locations shown on the construction drawings;
 - 2. top of compacted clay liner;
 - 3. top of leak detection system (LDS);
 - 4. top of leachate collection system (LCS);
 - 5. top of impacted protective layer and non-impacted protective layer within impacted runoff catchment area and intercell berm;
 - 6. top of protective clay layer in Area 1;
 - 7. top of impacted select layers;
 - 8. top of impacted material placement;
 - 9. top of select impacted material placement;
 - 10. top of contouring layer;
 - 11. top of compacted clay cap;
 - 12. top of protective clay layer in Area 4;
 - 13. top of cover drainage layer;
 - 14. top of biointrusion barrier with choking layer;
 - 15. top of granular filter;
 - 16. top of vegetative soil layer; and
 - 17. top of topsoil layer.
- C. Submit electronic files and two hard copies of the survey notes, sketches, and drawings for the following surveys to the Construction Manager within one week of performance:
 - 1. preliminary surveys;
 - 2. intermediate surveys;
 - 3. written statement and surveys for conformance checks and "red-line" drawings;
 - 4. surveys prior to end of construction season and/or winter shutdown;
 - 5. survey at completion of impacted material excavation specified in Section 02205 of the following:
 - a. above Waste Acceptance Criteria (WAC) material;

- b. above Final Remedial Level (FRL) material;
 - c. Resource Conservation and Recovery Act (RCRA) hazardous waste;
 - d. underground storage tanks and/or associated soil; and
 - e. Hazardous Waste Management Units (HWMUs);
 - 6. survey at completion of the Contract;
 - 7. measurement and payment surveys; and
 - 8. final surveys.
- D. On request by the Construction Manager, submit documentation verifying accuracy of survey work.
- E. Upon completion of the survey work, provide the Construction Manager the original field notes, layout, computations, signed and sealed sketches and drawings in Microstation 95 ".dgn" format or electronic files in other format approved by the Construction Manager.
- F. One complete set of final "red-line" drawings, sketches, and survey notes signed and sealed by a Land Surveyor licensed in the State of Ohio shall be submitted to the Construction Manager within 15 days of completion of the Contract. Drawing and sketch format shall be Microstation 95 ".dgn" or electronic files in other format approved by the Construction Manager. Survey notes shall include a point listing with coordinates, elevation, and description.

1.06 PROJECT RECORD DOCUMENTS

- A. Maintain on site, a complete, accurate log documenting survey work as it progresses.
- B. Maintain on-site, a plan showing survey control points, and benchmarks with coordinates and elevations.
- C. Maintain on-site, an accurate and current set of marked-up "red-line" drawings showing "as-built" conditions. "As-built" conditions shall be marked-up on "red-line" drawings within one week of completion of the respective construction activity.

1.07 HEALTH AND SAFETY REQUIREMENTS

- A. Environmental health & safety/training requirements shall be in accordance with Part 8 of the Contract Documents.

1.08 CONTRACTOR'S QUALITY ASSURANCE

- A. Contractor's quality assurance requirements shall be in accordance with Part 9 of the Contract Documents.

PART 2 PRODUCTS

2.01 MATERIALS AND SURVEY INSTRUMENTS

- A. Provide materials as required to properly perform the surveys, including, but not limited to, personal protective equipment, instruments, tapes, rods, measures, mounts and tripods, stakes and hubs, nails, ribbons, other reference markers, and all else as required.
- B. The survey instruments used for this work shall be precise and accurate to meet the needs of the project. Survey instruments should be capable of reading to a precision of 0.001 feet and with a setting accuracy of ± 8 seconds.

PART 3 EXECUTION

3.01 GENERAL

- A. Maintain accurate and complete notes of surveys:
 - 1. Handwritten survey field notes and information shall be documented. A copy of the numbered, dated, and signed documentation shall be given to the Construction Manager weekly or upon request by the Construction Manager. Survey notes shall be legibly recorded. Notation shall be consistently applied to survey work. The stake marking format and the document notations shall be compatible. Identify survey benchmarks on the field notes, sketches, and drawings.
 - 2. Electronically collected field survey information shall be stored, for retrieval and submittal if requested by the Construction Manager, during the period of performance of the Contract.
 - a. Electronic format for printed output of data collector field survey notes shall be compatible with the approved field notation format.
 - b. Electronic format for printed output of data collector survey work shall be compatible with the Contractor's computer equipment and software specified in this Section for verifying and checking the work. A copy of the data disk shall be submitted to the Construction Manager monthly.
 - 3. Submit electronic file and two hard copies of above information when requested by the Construction Manager
- B. During construction, survey notes shall be retained by the Contractor and Land Surveyor.

- C. Perform surveys for conformance checks specified in this Section. Contractor shall submit a written statement with conformance surveys certifying compliance of the preceding layer thickness, limits, and grades to the Construction Manager.
- D. The precision of horizontal and vertical controls shall meet or exceed Third-Order Class I and Third-Order accuracies, respectively, as defined by NGS Standards.
- E. Conformance check surveys for elevation and for horizontal coordinates shall be recorded to the nearest 0.01 feet and for angles shall be to the nearest 20 seconds.
- F. Measurement and payment surveys for elevation and for horizontal distances shall be recorded to the nearest 0.1 feet and 0.05 feet, respectively.
- G. Final "red-line" drawings and sketches shall be signed and sealed for method and accuracy of work and sealed by the Land Surveyor.
- H. Perform construction layout surveys in advance of scheduled construction activities. At completion of a survey, provide a copy of the field notes, drawings, or sketches to the Construction Manager for review. The Contractor shall allow the CQC Consultant and/or Construction Manager three working days for review of conformance surveys. The Contractor shall be responsible for rework and/or construction delays caused by survey or staking errors.
- I. Set grade and slope stakes required for construction activities as the work progresses. Staking shall be in accordance with accepted surveying practices, provisions herein, and subject to Construction Manager review. Set fine grade stakes on all surfaces for which the plans show a definite grade line. Grade stakes shall not be permitted on soil layers overlying any geosynthetic material within 12 inches of the geosynthetic material or on the contouring layer and first lift of compacted clay cap.
- J. Verify pipe alignment and elevation. The Contractor shall:
 - 1. check layout and elevation of pipe embedment fill prior to pipe placement;
 - 2. check pipe alignment during placement and backfill; and
 - 3. verify alignment and elevation at top of pipe after pipe has been backfilled to top of pipe at a maximum interval of 25 feet.
- K. Upon completion of the work, the Contractor shall provide the Construction Manager with original survey field notes, layouts, computations, and electronic files, binders containing electronic file information and one copy each of electronic files specified in this Section.
- L. Protect benchmarks and survey control points. Replace disturbed survey control points and benchmarks at no additional cost.

- M. Establish temporary survey control points to support construction work activities.
- N. Survey control points, accuracy, and documentation:
 - 1. Record the following information in survey notebooks for each control point established:
 - a. designation of control point;
 - b. coordinates based on State Planar North American Datum (NAD) 1983 Ohio South;
 - c. elevations based on National Geodetic Vertical Datum (NGVD);
 - d. date of establishment;
 - e. description and sketch of the control point location; and
 - f. control points referenced to a minimum of three features that can be seen from the control point.
 - 2. Document survey work in the fieldbooks using the format and procedures described below:
 - a. title and consecutive fieldbook number on the front cover;
 - b. consecutively numbered pages;
 - c. table of contents, indicated by survey task, on the first numbered page;
 - d. legend indicating symbols used in survey notes;
 - e. names of survey team for each task;
 - f. notes on weather, equipment, etc.;
 - g. date and time on each page to indicate when work was recorded;
 - h. notes in a uniform character such that they can be interpreted and used by anyone with survey knowledge; and
 - i. description and/or sketches of the existing survey control used.
- O. Provide hardware and software to download data to Fluor Fernald computers as approved by Engineering Manager.

3.02 SUPPORT SURVEYS

- A. Preliminary Surveys:
 - 1. Verification of the Existing Conditions:
 - a. Prior to the start of clearing and earthwork activities, verify the accuracy of the existing conditions shown on the Construction Drawings and Reference Drawings. Immediately notify the Construction Manager in writing of deviations from the existing conditions indicated on the Construction Drawings and Reference Drawings that affect construction cost and/or schedule.
 - 2. Verify the existing benchmarks, structures, utilities, wells, topography, surface-water management and erosion control measures, construction safety and radiological-control fences, sedimentation basins and appurtenances, drainage

features, and existing stockpiles of materials and quantities shown on the Construction Drawings, Reference Drawings, or specified in the Contract. Notify the Construction Manager of any differences or conflicts with work included in this Contract.

3. Verify Mid-Valley Pipeline Easement as shown on the Construction Drawings adjacent to battery limits. Stake western limits of easement at 50-foot intervals.
4. Establish construction limits required for installation of the construction safety fence and radiological-control fence specified in Section 02200.
5. Establish location for the installation of the surface-water management and erosion control measures specified in Sections 02270, 02271, and 02275.
6. Clearing Limit Staking: Stake clearing limits specified in Section 02110.
7. Alignment and Existing Ground Staking: Following clearing operations and before stripping operations begin, preliminary locations of alignments and/or baseline of project features shall be established. Perform topographic surveys to describe original ground features before stripping or excavation begins. The distance between grid points shall not exceed 50 feet, and all breaks shall be noted.
8. Earthwork Staking: Staking for excavation and fill limits shall establish the exterior limits of excavations and fills. The maximum staking interval shall be 50 feet. Stakes shall be prominently noted with description of point, vertical distance to design elevation, and offset distance as applicable.
9. Perform additional surveys required for the layout of other construction activities.
10. Prior to construction activities in remedial excavation areas specified in Sections 02205 and 02207, perform topographic surveys of above-WAC and impacted runoff catchment area (IRCA) stockpiles, staging areas, excavation areas, and concrete crushing support areas at minimum 50-foot intervals with additional points as follows:
 - a. grade breaks;
 - b. points of horizontal curvature and tangency;
 - c. edge and corners of concrete or asphalt pads, slabs, catch basins, and manholes;
 - d. above-grade obstructions (e.g., fire hydrants, utility poles, handrails, etc.); and
 - e. ditches, channels, and depressions.In addition, spot check slab elevations as indicated on applicable building foundation reference drawings. This includes basement, pit, sump, and other below-grade slab elevations.
11. Prior to area isolation trench excavation specified in Section 02207, survey the trench location and stake the centerline of the trenches.
12. Initial limits of excavation specified in Section 02207 shall be surveyed and staked after the completion of excavation of area isolation trenches.

13. The centerline of the completed area isolation trench specified in Section 02207 shall be surveyed with reference to the permanent trench monuments and staked after the completion of trenching. Provide redline mark-ups showing where the trench deviates from the design location by more than the maximum lateral tolerance specified in Section 02207. The Contractor will identify the area isolation trench using a rope boundary. The rope will be a color other than orange, yellow, or magenta.

B. Intermediate Surveys:

1. Perform surveys during progress of the construction activities to verify the accuracy of work and as directed by the Construction Manager. These surveys include, but are not limited to, surveys of the subgrade excavation; compacted clay liner and cap; LDS, LCS, and cover drainage layers; protective layer; contouring layer; biointrusion barrier layer; granular filter; vegetative soil layer; topsoil; LDS and LCS piping; horizontal monitoring wells; select impacted material layer; impacted material layer; and other surveys directed by the Construction Manager.
2. Perform surveys for the impacted material placement and stockpiles as follows:
 - a. Survey the locations and surface of impacted material placement when the category of impacted material changes in any given grid. Survey the locations and surface of the previous layers prior to changing impacted material category within the grid.
 - b. Survey the locations and surface of completed impacted material lifts in any grid where placement occurs at or near the end of the workday.
 - c. Survey the impacted material placed in the OSDF every week and within one working day of the last day of the month. This survey shall include locations, elevations, category of impacted materials, and pertinent information in ASCII format necessary to develop a 3-D topographic computer generated surface.
 - d. Perform interim surveys of impacted material stockpiles or other areas designated by the Construction Manager. This survey shall include volume (CY) remaining in the stockpiles or in designated areas.
 - e. Survey the impacted material surface in the OSDF, including the cell access ramps, at the completion of the construction season activities.
3. Perform surveys for measurement and periodic progress payment as specified in this Section.
4. Perform surveys during progress of impacted material excavations specified in Section 02205 to confirm limits of the excavation.
5. Perform survey if either the unsaturated sands and gravel of the Great Miami Aquifer (GMA) are encountered or excavation has reached the GMA elevation as indicated on the Construction Drawings.

6. Perform surveys after the installation of the first 2 feet of GMA plug placement to confirm 2 feet thickness.
 7. Perform surveys upon restoration of the minimum 5-foot protective cover over the unsaturated sands and gravel of the GMA.
 8. Perform surveys when establishing new Special Material Transfer Areas.
 9. Prior to obtaining GMA plug material as specified in Section 02206, perform topographic survey and establish work limits of designated borrow areas.
 10. Perform survey at the completion of each supplemental excavation.
- C. Final Surveys:
1. Final topography shall be surveyed at nominal 50-foot intervals. Additionally, the following points shall be surveyed as applicable:
 - a. grade breaks;
 - b. points of horizontal curvature and tangency; and
 - c. points of stationing equation.
 2. Structures: Survey structure centerlines or building lines so that the orientation, position, limits, and foundation elevation(s) are positively identified.
 3. Ditches and Channels: Survey ditches, channels, and culverts as specified in Sections 02270, 02275, and 02721.
 4. Limits of Final Excavations: Survey limits of final impacted material excavations.
 5. Pipes: Utility pipes shall be surveyed at nominal 25-foot intervals at the top of pipe. Surface-water management pipes shall be surveyed at inlet and outlet inverts and along perimeter of riprap protection.

3.03 SURVEYS FOR MEASUREMENT AND PAYMENT

- A. Perform surveys for periodic progress payments and final payment to determine quantities of work and percent of completed work. Quantities to be measured and measurements for quantities shall be as specified in Part 6 of the Contract Documents.
- B. Calculate and certify quantities of work and submit survey notes and calculations to the Construction Manager for review, evaluation, and payment.

3.04 SURVEYS FOR CONFORMANCE CHECKS AND "RED-LINE" DOCUMENTS

- A. Survey the following to verify the locations, lines, and grades achieved during construction for conformance checks and "red-line" documents:
 1. for berms, roads, ditches, and other earthwork specified in Sections 02200, 02206, 02230, and 02270:
 - a. original grade surface;
 - b. compacted surface of cut slopes; and
 - c. finished grade surface;

2. for culverts and other surface-water management and erosion control structures specified in Sections 02270 and 02721:
 - a. original grade surface;
 - b. pipe inverts; and
 - c. finished grade surface including riprap protection at inverts;
3. for the subgrade specified in Section 02200:
 - a. prepared subgrade surface;
4. for the trenching and backfilling specified in Section 02215;
 - a. bottom of the liner system anchor trench and pipe trench and top of finished compacted backfill; and
 - b. pipes and culverts;
5. for the compacted clay liner specified in Section 02225:
 - a. finished compacted clay liner surface;
6. for the liner penetration boxes specified in Section 13005 and shown on the Construction Drawings:
 - a. original grade surface; and
 - b. top of liner penetration boxes;
7. for the LDS, LCS, and cover drainage layers, and piping specified in Sections 02605 and 02710:
 - a. finished grade surface;
 - b. horizontal monitoring wells; and
 - c. piping system, including location of each joint;
8. for the granular filter specified in Section 02712:
 - a. finished granular filter surface;
9. for the compacted clay cap specified in Section 02225:
 - a. prepared top of non-impacted contouring layer surface; and
 - b. finished compacted clay cap surface;
10. for the geomembrane liner specified in Section 02770:
 - a. anchor trench;
 - b. top and bottom of side slopes;
 - c. seam intersections;
 - d. repairs; and
 - e. location of destructive testing;
11. for the non-impacted protective layer specified in Section 02240:
 - a. finished protective layer surface;
12. for impacted material placement surface specified in Section 13010:
 - a. top of protective layer surface;
 - b. top of select impacted material layers (liner and final cover system) surfaces;
 - c. top of final impacted material surface (prior to placement of select impacted material layer in final cover system); and

- d. Category 5 material placement per specialized placement plans as specified in the IMP Plan.
 - 13. for biointrusion barrier specified in Section 02280:
 - a. finished biointrusion barrier surface;
 - 14. for vegetative soil layer specified in Section 02250;
 - a. finished vegetative soil layer surface;
 - 15. for topsoil specified in Section 02920;
 - a. top of topsoil surface;
 - 16. for chain-link fences and gates specified in Section 02831:
 - a. location and alignment;
 - 17. for the borrow area specified in Section 13000;
 - a. test pits used for pre-conformance testing; and
 - b. finished grades of interim restoration of borrow subareas;
 - 18. centerlines at nominal 50-foot intervals and at points of intersection of area isolation trenches specified in Section 02207.
- B. Drawings and sketches for the items described in this Section shall include the following:
- 1. North arrow, graphical scale, title block, and legend;
 - 2. Northing and Easting grid lines;
 - 3. spot grade location and elevation on plan including list of coordinates with point number, northing, easting, and elevation in table format;
 - 4. 1-foot contour lines;
 - 5. location of structures;
 - 6. labeled components;
 - 7. for geomembrane liner and cap plans:
 - a. all seams;
 - b. panel identification numbers;
 - c. location of top of slope, toe of slope, anchor trench, and limits of geomembrane;
 - d. repair location and identifying number; and
 - e. destructive testing location and identifying number; and
 - 8. for pipe profiles:
 - a. original grades with stationing;
 - b. final grades with stationing; and
 - c. pipe with inverts, slopes, pipe material, pipe size, and length of pipe.
- C. Perform conformance checks and "red-line" surveying immediately upon completion of a given installation or excavation activity to facilitate progress and avoid delaying commencement of the next installation. Provide the following minimum spacings and locations for survey points (additional survey points may be required if field conditions warrant):

1. surfaces with gradients less than 10 percent, survey on a square grid spaced not wider than 50 feet;
2. on slopes greater than 10 percent, a square grid spaced not wider than 50 feet shall be used, but in any case, a line at the crest and toe of the slope shall be taken; if the slope distance is greater than 50 feet, a midpoint slope survey point is required;
3. a line of survey points spaced not more than 50 feet apart shall be taken along any slope break (this will include the inside edge and outside edge of any bench on a slope);
4. a line of survey points spaced not more than 25 feet apart shall be taken at the top of any pipes or other appurtenances; and
5. at the corners and midpoints of the top and bottom of slope breaks for liner penetration areas in cells.

[END OF SECTION]

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OSDF PHIV-SPEC REV 0
Section 02200: Earthwork

SPECIFICATION COVER SHEET

SPECIFICATION SECTION: 02200 TITLE: EarthworkSpecifications By: Signature
(Cognizant Engineer)Michael J. Monteleone, P.E.
and Title Associate17 Aug 01
Date

Scope and Format

Checked By: Signature
(Checker)Michael J. Monteleone, P.E.
and Title Associate17 Aug 01
Date

Detailed Requirements

Checked by: Signature
(Checker)David K. Phillips
and Title Senior Project Engineer17 Aug 01
DateOverall Review By: Signature
(PDP)J.F. Beech, Ph.D., P.E.
and Title Principal20 Aug 2001
DateApproved by: Signature
(DTL)J.F. Beech, Ph.D., P.E.
and Title Principal20 Aug 2001
Date

Record of Revision (Number and initial all revisions)

Rev. No.	Reason	Date	By	Checked	Approval
0	Certified for Construction	20 August 01	mon	DKP	JFB
1	Revisions from Phase III DCNs	13 March 02	DKP	DKP	JFB

SECTION 02200

EARTHWORK

PART 1 GENERAL

1.01 SCOPE

- A. This Section includes site preparation including construction safety fence and radiological-control fence, surface-water management and erosion control, excavation, dewatering, stockpiling, subgrade and top of contouring layer preparation, compacted fill, and clayey rockfill.

1.02 RELATED SECTIONS AND PLANS

- A. Section 02100 - Surveying
- B. Section 02110 - Clearing, Grubbing, and Stripping
- C. Section 02150 - Traffic Control
- D. Section 02205 - Impacted Material Excavation
- E. Section 02206 - Earthwork for Remediation
- F. Section 02215 - Trenching and Backfilling
- G. Section 02225 - Compacted Clay Liner and Cap
- H. Section 02230 - Road Construction
- I. Section 02240 - Non-Impacted Protective and Contouring Layers
- J. Section 02250 - Vegetative Soil Layer
- K. Section 02270 - Surface-Water Management and Erosion Control
- L. Section 02271 - Riprap
- M. Section 02280 - Biointrusion Barrier
- N. Section 02605 - High-Density Polyethylene (HDPE) Pipes and Fittings
- O. Section 02710 - Granular Drainage Material

- P. Section 02712 - Granular Filter Material
- Q. Section 02714 - Geotextiles
- R. Section 02721 - Culverts
- S. Section 02831 - Chain-Link Fences and Gates
- T. Section 02920 - Topsoil
- U. Section 02930 - Vegetation
- V. Section 13000 - Borrow Area Management
- W. Section 13005 - Liner Penetration Boxes
- X. Section 13010 - Impacted Materials Placement
- Y. Construction Quality Assurance (CQA) Plan
- Z. Part 6 - Statement of Work
- AA. Part 8 - Environmental Health & Safety/Training Requirements
- BB. Part 9 - Quality Assurance Requirements

1.03 REFERENCES

- A. Latest version of American Society for Testing and Materials (ASTM) Standards:
 - 1. ASTM D 698. Test Method for Laboratory Compaction Characteristics of Soil Using Standard Effort (12,400 ft-lbf/ft³ (600 kN-m/m³)).
 - 2. ASTM D 2487. Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System).
- B. Reference Reports addressing On-Site Disposal Facility (OSDF) and borrow area site subsurface conditions:
 - 1. "Geotechnical Investigation Report, On-Site Disposal Facility" [Parsons, 1995]. This report contains geotechnical data for the subsurface soils in the OSDF area.
 - 2. "Disposal Facility Pre-Design Geotechnical Investigation, Soil Investigation Data Report, CERCLA-RCRA Unit 2" [Science Applications International Corporation, 1995]. This report presents geotechnical data for the subsurface soils in the OSDF area.

3. *"Geotechnical Data and Evaluation Report for East and South Field Borrow Areas"* [Parsons, 1996]. This report contains geotechnical data for the subsurface soils in the borrow area.

1.04 SUBMITTALS

- A. Submit to the Construction Manager for review a Contractor's Earthwork Work Plan within 30 calendar days from Notice to Proceed. The Contractor's Earthwork Work Plan shall include, at a minimum:
 1. list of equipment and description of construction methods proposed for the scope specified in this Section and in Sections 02110, 02206, 02215, 02225, 02230, 02240, 02250, 02271, 02280, 02605, 02710, 02712, 02714, 02721, 02831, 02920, 02930, 02940, and 13005;
 2. location of equipment service area and fueling station;
 3. excavation and trenching dewatering methods and techniques;
 4. methods for removal of visible rock particles larger than specified from the material for compacted fill and clayey rockfill specified in this Section and compacted clay liner and cap specified in Section 02225;
 5. coordination of survey requirements for the site work;
 6. verification of the existing conditions and material stockpiles;
 7. stockpile management plan including surface-water management and erosion control, stockpiling by type of material, stockpile maintenance, stockpile removal and relocation, and site grading and stabilization;
 8. coordination of earthwork activities with Contractor's Surface-Water Management and Erosion Control Work Plan specified in Section 02270;
 9. schedule for site work activities;
 10. water supply system including location, type, and size of water tank, water distribution system and equipment for dust control, construction and wheel wash system;
 11. plan and measures for cold weather at temperatures below 32 degrees Fahrenheit site work activities;
 12. installation and maintenance of construction safety fence and radiological-control fence;
 13. coordination with Traffic Control Plan specified in Section 02150;
 14. location of construction laydown area(s);
 15. locations of stockpiles for material generated from clearing, grubbing, and stripping operations;
 16. layout and typical cross sections of roads within the Contractor's work area;
 17. construction site access and haul road layout;
 18. construction utilities layout including construction power and water;
 19. description of methods for installation and removal of trench supports; and

20. coordination with the Contractor's Quality Assurance Work Plan in accordance with Part 9 of the Contract Documents.

1.05 HEALTH AND SAFETY REQUIREMENTS

- A. Environmental health & safety/training requirements shall be in accordance with Part 8 of the Contract Documents.

1.06 CONTRACTOR'S QUALITY ASSURANCE

- A. Contractor's quality assurance requirements shall be in accordance with Part 9 of the Contract Documents.

PART 2 PRODUCTS

2.01 MATERIALS

- A. Obtain fill material for compacted fill from OSDF cell excavation, trenching, and stockpiles approved by the Construction Manager. Obtain additional fill material for compacted fill, if required, from the on-site borrow area indicated on the Construction Drawings. Borrow area management shall be in accordance with Section 13000.
- B. Fill material for compacted fill and trench backfill shall be free of debris, foreign objects, large rock fragments, organics, and other deleterious materials. Visible rock particles shall be maximum dimension of 5 inches for 8-inch \pm 1-inch thick loose lifts and 2 inches for 4-inch \pm 1-inch thick loose lifts. Material for compacted fill shall conform to GC, SC, SM, ML, CL, or CH according to the Unified Soil Classification System (per ASTM D 2487).
- C. Clayey rockfill: The reject material from clay screening operations may be substituted for material for compacted fill below base aggregate elevations in the impacted material haul roads, cell access ramps, access corridor located outside the perimeter berm baseline, and borrow area haul road; for interim restoration in borrow area; and other fill areas outside the OSDF perimeter berm baseline. Use of clayey rockfill as specified in this Section shall be approved by the Construction Manager.
- D. Construction water for moisture conditioning compacted fill shall be obtained from the on-site water source shown on the Construction Drawings.
- E. Construction safety fence and radiological-control fence for activities with duration less than 30 calendar days shall be orange, high-density polyethylene, 4 feet in height, opening size approximately 4 inches by 1 inch, minimum tensile strength of 2000

pounds per foot of width. Where used to delineate radiologically controlled areas (RCAs), no fencing is required. Contractor shall install posts and Fluor Fernald will provide and install yellow/magenta rope. Posts shall be T-shaped (T-post), 1-1/2 inch by 1-1/2 inch, 3/16 inch thick by 6 feet long, and made of steel or as approved by the Construction Manager.

- F. Furnish construction safety fence and radiological-control fence for activities with a duration greater than 30 calendar days made of galvanized steel welded wire fabric, 2 inch by 4 inch mesh, 4 feet in height, 12-1/2 gauge, or equivalent approved by the Construction Manager. Posts for the fence material shall be 6 feet long and made of steel. Install posts at spacing recommended by the Manufacturer's installation procedures and as required to prevent sagging.
- G. Contractor shall furnish and install signs for construction safety fence in accordance with Part 8 of the Contract Documents.
- H. Signs for radiological-control fence shall be furnished and installed by Fluor Fernald, Inc.

2.02 EQUIPMENT

- A. Furnish equipment to perform work specified in this Section.
- B. Furnish equipment to achieve required compaction specified in this Section.
- C. Furnish hand compaction equipment, such as walk-behind padfoot compactors, hand tampers, or vibratory plate compactors, for compaction in areas inaccessible to large compaction equipment.
- D. Furnish water tank trucks, pressure distributors, or other equipment designed to apply water uniformly and in controlled quantities at variable surface widths to provide the required in-place moisture content and to prevent drying of soil surfaces.
- E. Furnish equipment such as scarifiers, disks, spring tooth or spike tooth harrows, earth hauling equipment, and other equipment as required for earthwork construction.

PART 3 EXECUTION

3.01 GENERAL

- A. Verify existing conditions in accordance with Section 02100.

- B. Perform construction activities in such a manner that equipment operating in the RCAs do not operate in non-RCAs. Equipment operating in RCAs shall be washed by the Contractor; and radiologically surveyed and released by Fluor Fernald, Inc. prior to exiting for use in non-RCAs.
- C. Install surface-water management and erosion controls in accordance with Section 02270.
- D. Dust control shall be in accordance with Part 6 of the Contract Documents.

3.02 SITE PREPARATION

- A. Install construction safety fence and radiological-control fence at construction limits and limits of the RCAs in accordance with the Contractor's Earthwork Work Plan and Part 8 of the Contract Documents. Relocate construction safety fence and radiological-control fence as shown on the Construction Drawings or as approved by the Construction Manager. Provide construction safety fence as shown on Construction Drawings. Signs and barricades around trenches, stockpiles, and excavated areas shall be in accordance with Part 8 of the Contract Documents.
- B. Maintain and repair construction safety fence and radiological-control fence for the duration of the Contract. Fencing shall be maintained so as to minimize vertical sagging.
- C. Install, maintain, and inspect surface-water management and erosion controls in accordance with Section 02270.
- D. Prior to earthwork activities, perform clearing, grubbing, and stripping in accordance with Section 02110.
- E. Construct impacted material haul roads, cell access ramps, and access corridors in accordance with the Construction Drawings and Section 02230.
- F. Locate existing manholes, drop inlet structures, monitoring wells, piezometers, lysimeters, utilities, and other subsurface structures in the work area. Protect structures and utilities during earthwork activities as indicated on the Construction Drawings and approved by the Construction Manager.

3.03 SURFACE-WATER MANAGEMENT AND EROSION CONTROL

- A. Install surface-water management and erosion controls in and around work areas in accordance with Section 02270.

3.04 EXCAVATION

- A. Excavate designated areas to the subgrade elevations or excavation limits shown on the Construction Drawings. Stockpile excavated material in the designated stockpile area shown on the Construction Drawings or at locations approved by the Construction Manager.
- B. Excavate material within the excavation limits, including rock encountered, regardless of type, character, composition, and condition. Remove clay pipe tile subdrain system when encountered in accordance with Section 02205. Place clay pipe subdrain section in the OSDF constructed cells in accordance with Section 13010.
- C. Blasting, including use of explosives or explosive devices, shall not be permitted.
- D. Remove and relocate impacted material encountered during excavation in accordance with Section 02205. Impacted material meeting OSDF Waste Acceptance Criteria (WAC) shall be placed in accordance with Section 13010. Impacted materials exceeding OSDF WAC shall be disposed of in accordance with Section 02205.
- E. Minimize sloughing and caving of excavations. Over-excavate and fill areas of excavations that cave or slough with compacted fill in accordance with this Section.
- F. Over-excavate abandoned monitoring wells, borings, utilities, and lysimeters within the OSDF perimeter baseline shown on the Reference Drawings to a depth of 3 feet below subgrade elevation. Well casings, concrete, and grout shall be excavated in accordance with Section 02205 and placed in the OSDF constructed cells in accordance with Section 13010. Before removal of the existing well casings, Construction Managers will verify closure of the existing wells. Fill to subgrade elevation with compacted fill in accordance with this Section.
- G. Do not remove soil from the site or dispose of soil included in this Contract except as approved in writing by the Construction Manager.
- H. Perform activities in such a manner that hauling equipment transporting non-impacted materials do not operate on roads used to haul impacted material. Equipment driven on roads used to haul impacted material or in an impacted area shall be washed by Contractor, and radiologically surveyed and released by Fluor Fernald, Inc. prior to being used for earthwork activities in non-impacted areas.
- I. Perform activities in such a manner that earthwork and hauling equipment working in contamination areas do not cross into certified areas.

- J. Remove the existing Rerouted North Entrance and North Entrance Road pavement within the limits shown on the Construction Drawings and haul and place in OSDF constructed cells in accordance with Section 13010. Existing North Entrance Road pavement section consists of 6 inches of asphaltic concrete over 6 inches of crushed rock. Existing rerouted North Entrance Road pavement section consists of 12 inches of asphalt concrete over 8 inches of crushed rock. Existing rerouted North Entrance Road pavement section consists of 12 inches of asphaltic concrete over 8 inches of crushed rock. Excavate 2 feet below bottom of pavement elevation and road shoulder in accordance with Section 02205 and place in accordance with Section 13010, unless otherwise directed by the Construction Manager.
- K. Stabilize disturbed areas in accordance with Section 02930.

3.05 EXCAVATION DEWATERING

- A. Anticipate seepage of groundwater into and accumulation of surface-water runoff in excavations. Manage groundwater and surface-water runoff in excavations in accordance with this Section and Section 02270.
- B. Collect water that accumulates in the excavation in a toe drain, or other suitable sump, and pump to the former production area stormwater drain control system, the leachate transmission system, or other locations as directed by the Construction Manager.
- C. Prevent surface-water runoff from adjacent areas from entering the excavation in accordance with Section 02270.

3.06 STOCKPILING

- A. Stockpile excavated soils in the stockpile areas shown on the Construction Drawings or as directed by the Construction Manager.
- B. Construct stockpiles no steeper than 3H:1V (horizontal:vertical), grade to drain, seal by tracking perpendicular to the slope contours with a dozer, and dress daily during periods when material is taken from or added to the stockpile.
- C. Install surface-water management and erosion control measures at the stockpile areas in accordance with Section 02270. Stabilize stockpiles in accordance with Section 02930.

3.07 SUBGRADE AND TOP OF CONTOURING LAYER PREPARATION

- A. Subgrade and top of contouring layer material shall be free of debris, foreign objects, organics, and other deleterious materials.

- B. In the event saturated subgrade is encountered, localized sumps shall be constructed to facilitate removal of water. Manage removed water in accordance with this Section.
- C. Perform subgrade and top of contouring layer proofrolling by driving a loaded dump truck with minimum loaded weight of 20 tons and minimum weight of 10 tons per axle or other pneumatic-tired vehicle back and forth across the area to be prepared to confirm the firmness of subgrade and top of contouring layer surface. Overlap the passes such that one set of tires on each pass runs between the two sets of tire tracks from the previous pass. Soils shall not exhibit pumping or develop ruts more than 2 inches in depth. Rutting, larger than 2 inches, shall be scarified in accordance with this Section and regraded with compacted fill material or non-impacted contouring layer material to meet the proposed subgrade or top of contouring layer elevations.
- D. Subgrade for the compacted clay liner and top of contouring layer shall be scarified in accordance with Section 02225. At other locations where compacted fill is to be placed, prepare the subgrade by scarifying to a depth of 2 inches using the equipment identified in this Section.
- E. In areas where unsuitable soils are encountered, remove and replace the soil to a minimum depth of 1 foot below the proposed subgrade elevation. Remove unsuitable subgrade to an additional depth if necessary to obtain a suitable soil surface for subsequent fill placement. Removal of unsuitable soils to additional depth shall be as approved by the Construction Manager. Suitable soil surface exhibiting pumping or developing ruts more than 2 inches in depth shall be removed to a minimum depth of 1 foot or dried in place by a method approved by the Construction Manager. Fill areas from which subgrade has been removed with compacted fill in accordance with this Section. Compact the fill material to at least 95 percent standard Proctor maximum dry unit weight as determined by ASTM D 698. Compact the uppermost lift of compacted fill beneath road and access corridor alignments to a minimum 98 percent of the standard Proctor maximum dry unit weight as determined by ASTM D 698.
- F. In excavations or other areas where water accumulates, implement measures to remove the water in accordance with this Section. Maintain the subgrade surface free of standing water and in a firm condition to meet the proofrolling requirements of this Section. Maintain dewatered areas in this condition until overlying construction is complete.
- G. Manage surface-water runoff or runoff in accordance with Section 02270.

3.08 COMPACTED FILL

- A. Use fill material that meets the material requirements of this Section. Place the fill material to the limits and grades shown on the Construction Drawings.

- B. Place fill material on surfaces which are free of debris, branches, vegetation, mud, ice, or other deleterious materials.
- C. Place fill material in loose lifts with a thickness of 8 inches \pm 1 inch. In areas where compaction is to be performed using hand-operated equipment, place the fill material in loose lifts with a thickness of 4 inches \pm 1 inch.
- D. Remove visible rock particles with a maximum dimension larger than 5 inches for 8-inch \pm 1-inch thick loose lifts. For 4-inch \pm 1-inch thick loose lifts, the maximum rock particle size shall be 2 inches.
- E. Prior to placing a succeeding lift of fill material over a previously compacted lift, thoroughly scarify the previous lift to a depth of 2 inches by discing, raking, or tracking with a dozer. Moisture condition the preceding lift in accordance with this Section if the moisture content of the surface of the preceding lift is not within the range of acceptable moisture contents specified in this Section.
- F. The trafficking of scarified surfaces by trucks or other equipment, except compaction equipment, is not permitted.
- G. The maximum acceptable soil clod size after processing is 3 inches. Reduce clod size by discing, raking, tracking with a dozer, using a soil stabilizer, or other means approved by the Construction Manager. Soil clumps, consisting of an agglomeration of 3-inch clods, or smaller, will not be considered a clod for purposes of this Section.
- H. Compact fill material in each lift to at least 95 percent of its standard Proctor maximum dry unit weight as determined by ASTM D 698. Compact fill at a moisture content within \pm 3 percentage points of the standard Proctor optimum moisture content as determined by ASTM D 698.
- I. Moisture condition the fill material to achieve the compaction requirements of this Section. Use a water spraying system for wetting. During wetting or drying, regularly disc, rake, or otherwise mix the material to thoroughly blend the moisture throughout the lift. Use discing, raking, or other appropriate methods to dry the material as required.
- J. Do not place frozen fill nor place fill material on frozen subgrade or previously placed compacted fill.
- K. Do not compact fill material at temperatures below 32 degrees Fahrenheit, unless authorized in writing by the Construction Manager.

- L. Do not place fill during periods of precipitation. Placement may occur during periods of misting or drizzle, but only if authorized by the Construction Manager.
- M. Rework compacted fill that does not meet the required compaction.

3.09 CLAYEY ROCKFILL

- A. Visible rock particles with maximum dimension greater than 12 inches shall be removed from the clayey rockfill and stockpiled for future use in areas designated by the Construction Manager. Clayey rockfill shall be placed in maximum 12-inch thick loose lifts and compacted with a minimum of four passes of a Caterpillar 815 compactor or approved equal. Final lift surface of compacted clayey rockfill material shall be proofrolled as specified in this Section. Any compacted clayey rockfill material exhibiting excessive pumping or rutting (ruts greater than 2 inches in depth) due to wet material or insufficient compaction shall be dried and recompact or removed from the fill. Clayey rockfill shall be used only as specified in this Section unless otherwise approved by the Construction Manager.

3.10 PERFORATIONS

- A. Perforations in the compacted fill, subgrade, and top of contouring layer resulting from survey stakes or other activities shall be backfilled with soil or bentonite mix specified in Section 02225. Perforations resulting from nuclear density tests and sand-cone or drive cylinder density tests will be filled by the CQC Consultant in accordance with Section 02225.

3.11 CONSTRUCTION QUALITY REQUIREMENTS

- A. CQC Consultant will perform soil conformance testing on compacted fill materials to confirm compliance with this Section. Conformance testing to be performed and minimum testing frequencies shall be in accordance with the Construction Quality Assurance (CQA) Plan. Provide equipment and labor to assist the CQC Consultant in obtaining conformance samples from excavations, stockpiles, and borrow areas. Identify source(s) and quantity of fill material required from each source for Construction Manager's approval at least 15 calendar days prior to use.
- B. CQC Consultant will monitor earthwork activities in accordance with this Section and the CQA Plan. CQC Consultant will provide documentation to the Construction Manager for the proofrolling of subgrade, top of contouring layer, and compacted clayey rockfill final lift surface.

- C. CQC Consultant will perform performance testing on compacted fill lifts to confirm compliance with this Section. The performance testing to be performed and minimum testing frequencies shall be in accordance with the CQA Plan.
- D. If CQC Consultant's tests indicate that any portion of the compacted fill does not meet the requirements of this Section, CQC Consultant will delineate the extent of the nonconforming area. Rework the nonconforming area until it meets the requirements of this Section.

3.12 SURVEY CONTROL

- A. Survey the locations, limits and grades of excavations, stockpiles, prepared subgrade, compacted fill, and compacted clayey rockfill in accordance with Section 02100.

3.13 TOLERANCES

- A. Perform the earthwork construction to within ± 0.3 feet of the grades indicated on the Construction Drawings except for subgrade for the compacted clay liner, top of contouring layer, access corridor, and roads for which earthwork construction shall be within -0.3 to $+0.1$ feet of the grades indicated.

[END OF SECTION]

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OSDF PHIV-SPEC REV 0
Section 02230: Road Construction

SPECIFICATION COVER SHEET

SPECIFICATION SECTION: 02230 TITLE: Road ConstructionSpecifications By: Signature
(Cognizant Engineer)Michael J. Monteleone, P.E.
and Title Associate17 Aug 01
Date

Scope and Format

Checked By: Signature
(Checker)Michael J. Monteleone, P.E.
and Title Associate17 Aug 01
Date

Detailed Requirements

Checked by: Signature
(Checker)David K. Phillips
and Title Senior Project Engineer17 Aug 01
DateOverall Review By: Signature
(PDP)J.F. Beech, Ph.D., P.E.
and Title Principal20 Aug 2001
DateApproved by: Signature
(DTL)J.F. Beech, Ph.D., P.E.
and Title Principal20 Aug 2001
Date

Record of Revision (Number and initial all revisions)

Rev. No.	Reason	Date	By	Checked	Approval
0	Certified for Construction	20 August 01	<u>msm</u>	<u>DIC</u>	<u>JFB</u>
1	Revisions from Phase III DCNs	13 March 02	<u>DIC</u>	<u>DIC</u>	<u>JFB</u>

SECTION 02230**ROAD CONSTRUCTION****PART 1 GENERAL****1.01 SCOPE**

- A. This Section includes impacted material haul roads, cell access ramps, access corridor, the Emergency Access Road, the Special Materials Transfer Area (SMTA), and other roads and areas as shown on the Construction Drawings to be surfaced with base aggregate.

1.02 RELATED SECTIONS AND PLANS

- A. Section 02100 - Surveying
- B. Section 02110 - Clearing, Grubbing, and Stripping
- C. Section 02150 - Traffic Control
- D. Section 02200 - Earthwork
- E. Section 02270 - Surface-Water Management and Erosion Control
- F. Section 02714 - Geotextiles
- G. Construction Quality Assurance (CQA) Plan
- H. Part 6 - Statement of Work
- I. Part 8 - Environmental Health & Safety/Training Requirements
- J. Part 9 - Quality Assurance Requirements

1.03 REFERENCE

- A. Latest version of Ohio Department of Transportation Construction and Material Specifications (Ohio DOT Specifications).

1.04 SUBMITTALS

- A. For each source of base aggregate material, submit the following to the Construction Manager for review within 30 calendar days from Notice to Proceed:
 - 1. the source of the materials along with written certification from the supplier that the material meets the material requirements of this Section; and
 - 2. certification shall include test results as required by Ohio DOT Specifications for base aggregate materials demonstrating that it meets the requirements of items from the Ohio DOT Specifications specified in this Section.
- B. Provide a list of equipment, description of construction methods, and other required information to perform the construction activities described in this Section with the Contractor's Earthwork Work Plan specified in Section 02200.

1.05 HEALTH AND SAFETY REQUIREMENTS

- A. Environmental health & safety/training requirements shall be in accordance with Part 8 of the Contract Documents.

1.06 CONTRACTOR'S QUALITY ASSURANCE

- A. Contractor's quality assurance requirements shall be in accordance with Part 9 of the Contract Documents.

PART 2 PRODUCTS

2.01 MATERIALS

- A. Furnish base aggregate material consisting of crushed carbonate stone or crushed gravel, free of organic matter and other deleterious materials, which meets the requirements of Items 304.02 and 703.04 (2) of the Ohio DOT Specifications for aggregate base.
- B. Furnish a geotextile separator meeting the requirements of Section 02714.
- C. Furnish materials for compacted fill or clayey rockfill meeting the requirements of Section 02200.
- D. Furnish road signs and other traffic controls in accordance with Section 02150.

2.02 EQUIPMENT

- A. Furnish equipment for construction of impacted material haul roads, cell access ramps, access corridors, the Emergency Access Road, the SMTA, and other roads shown on the Construction Drawings in accordance with the requirements of this Section.

PART 3 EXECUTION**3.01 GENERAL**

- A. Dust control during the performance of road construction activities described in this Section shall be in accordance with Part 6 of the Contract Documents.
- B. Install surface-water management and erosion controls in accordance with Section 02270.
- C. Perform clearing, grubbing, and stripping to the limits indicated on the Construction Drawings or identified by the Construction Manager, and in accordance with Section 02110 prior to any earthwork activity.

3.02 SUBGRADE PREPARATION

- A. Prepare subgrade for the road construction described in this Section in accordance with Section 02200.

3.03 GEOTEXTILE PLACEMENT

- A. Install the geotextile separator over the prepared subgrade in accordance with Section 02714.

3.04 BASE AGGREGATE

- A. Construct the base aggregate layer to the thickness, grades, and limits indicated on the Construction Drawings.
- B. Place the base aggregate material on top of the geotextile separator by end dumping and carefully spread using a track bulldozer. Do not operate equipment directly on the geotextile.
- C. Place the base aggregate in accordance with the requirements of Item 304.04 of the Ohio DOT Specifications.

- D. Compact the base aggregate in accordance with the requirements of Item 304.05 of the Ohio DOT Specifications.

3.05 CONSTRUCTION QUALITY REQUIREMENTS

- A. CQC Consultant will perform conformance testing on materials for compacted fill used for the construction described in this Section to establish compliance with this Section and Section 02200 as applicable. Conformance testing to be performed and minimum testing frequencies shall be in accordance with the Construction Quality Assurance (CQA) Plan.
- B. CQC Consultant will monitor road construction in accordance with this Section and the CQA Plan.
- C. CQC Consultant will perform performance testing on compacted fill and/or compacted clayey rockfill used for the construction described in this Section to establish compliance with this Section and Section 02200. Performance test requirements and minimum testing frequencies shall be in accordance with the CQA Plan.

3.06 SURVEY CONTROL

- A. Survey alignment and grades for roads, ramps, the SMTA, and corridor in accordance with Section 02100.

TOLERANCES

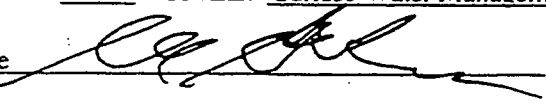
- A. Construct the base aggregate to within 0.0 to +0.1 feet of the thickness indicated on the Construction Drawings.
- B. Construct the impacted material haul roads, cell access ramps, access corridor, the Emergency Access Road, the SMTA, and other roads shown on the Construction Drawings to within ± 0.2 feet of the grades indicated on the Construction Drawings.


[END OF SECTION]

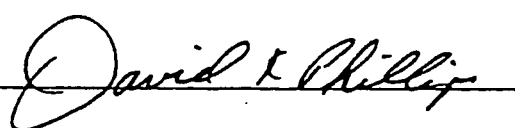
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SPECIFICATION COVER SHEET


SPECIFICATION SECTION: 02270 TITLE: Surface-Water Management and Erosion Control

Specifications By: Signature  17 Aug 01
(Cognizant Engineer) Date
Printed Name Michael J. Monteleone, P.E.
and Title Associate

Scope and Format
Checked By: Signature  17 Aug 01
(Checker) Date
Printed Name Michael J. Monteleone, P.E.
and Title Associate

Detailed Requirements
Checked by: Signature  17 Aug 01
(Checker) Date
Printed Name David K. Phillips
and Title Senior Project Engineer

Overall Review By: Signature  20 Aug 2001
(PDP) Date
Printed Name J.F. Beech, Ph.D., P.E.
and Title Principal

Approved by: Signature  20 Aug 2001
(DTL) Date
Printed Name J.F. Beech, Ph.D., P.E.
and Title Principal

Record of Revision (Number and initial all revisions)

Rev. No.	Reason	Date	By	Checked	Approval
0	Certified for Construction	20 August 01	msn	Dks	JFB
1	Revisions from Phase III DCNs	13 March 02	Dks	Dks	JFB

SECTION 02270

SURFACE-WATER MANAGEMENT AND EROSION CONTROL

PART 1 GENERAL

1.01 SCOPE

- A. This Section includes materials and placement of silt fence, erosion mat, check dams, construction entrances, diversions, ditches, channels, berms, and stabilization; and maintenance of sedimentation basins and surface-water management and erosion control measures.
- B. Surface-water management and erosion control for the impacted material placement shall be in accordance with Section 13010.

1.02 RELATED SECTIONS AND PLANS

- A. Section 02100 - Surveying
- B. Section 02200 - Earthwork
- C. Section 02240 - Non-Impacted Protective and Contouring Layers
- D. Section 02271 - Riprap
- E. Section 02275 - Surface Water Management and Erosion Control For Remediation
- F. Section 02721 - Culverts
- G. Section 02930 - Vegetation
- H. Section 13010 - Impacted Materials Placement
- I. Surface-Water Management and Erosion Control (SWMEC) Plan
- J. Construction Quality Assurance (CQA) Plan
- K. Part 6 - Statement of Work
- L. Part 8 - Environmental Health & Safety/Training Requirements
- M. Part 9 - Quality Assurance Requirements

1.03 REFERENCES

- A. Latest version of Ohio Department of Natural Resources (ODNR) Rainwater and Land Development Standards (ODNR Rainwater and Land Development Standards).

1.04 SUBMITTALS

- A. Submit to the Construction Manager for review within 15 calendar days from Notice to Proceed, Contractor's Surface-Water Management and Erosion Control Work Plan that shall be prepared in accordance with this Section, Section 02240, Section 02275, Section 13010, ODNR Rainwater and Land Development Standards, and the Surface-Water Management and Erosion Control (SWMEC) Plan, and shall include but not be limited to the following:
 - 1. descriptions of the surface-water management and erosion control measures to be implemented throughout the duration of the Contract;
 - 2. a list of equipment, description of methods, and other required information for installing and maintaining surface-water management and erosion control measures specified in this Section;
 - 3. drawings showing, in plan view, the location and sequencing of the surface-water management and erosion control measures and other required information for installation of surface-water management and erosion control measures;
 - 4. drawings showing details of the surface-water management and erosion control measures; and
 - 5. calculations supporting the selection and use of surface-water management and erosion control measures.
- B. Submit the following to the Construction Manager for review within 15 calendar days from Notice to Proceed:
 - 1. manufacturer's product data and recommended methods of installation for products used for surface-water management and erosion control measures; and
 - 2. certification from the supplier or Manufacturer that products meet the requirements of this Section.

1.05 HEALTH AND SAFETY REQUIREMENTS

- A. Environmental health & safety/training requirements shall be in accordance with Part 8 of the Contract Documents.

1.06 CONTRACTOR'S QUALITY ASSURANCE

- A. Contractor's quality assurance requirements shall be in accordance with Part 9 of the Contract Documents.

PART 2 PRODUCTS**2.01 SILT FENCE**

- A. Furnish silt fence with either woven or nonwoven geotextile conforming to ODNR Rainwater and Land Development standards. Silt fence shall:
 - 1. be woven geotextile consisting of slit films of polypropylene treated with ultraviolet light stabilizers, or nonwoven geotextile consisting of long chain polymeric filaments or polyester yarns;
 - 2. be inert to chemicals commonly found in soils and to hydrocarbons;
 - 3. be resistant to mildew, rot, insects, and rodent attack; and
 - 4. have geotextile and fence post properties and minimum dimensions in accordance with this Section and ODNR Rainwater and Land Development Standards.

2.02 EROSION MAT

- A. Furnish erosion mat which shall be a woven blanket-like fabric made of biodegradable yarn with the following material properties:
 - 1. Yarn Content: 100 percent jute except as indicated on Construction Drawings;
 - 2. Weight: Minimum 11.5 ounces per square yard;
 - 3. Open Area: 55 ± 10 percent; and
 - 4. Minimum Mesh Opening: 0.5 inches.
- B. Furnish erosion mat that will resist degradation for a minimum 6-month period after installation.
- C. Furnish erosion mat having a permissible velocity of 7 feet per second (fps).

2.03 STABILIZATION

- A. Materials for stabilization, including vegetation and crusting agent, shall be in accordance with Section 02930.

2.04 OTHER MATERIALS

- A. Riprap shall be in accordance with Section 02271.
- B. Culverts shall be in accordance with Section 02721.
- C. Materials for berms shall be as specified for compacted fill in Section 02200.
- D. Construction entrances shall be in accordance with ODNR Rainwater and Land Development Standards.

- E. Diversions and channels shall be in accordance with ODNR Rainwater and Land Development Standards.
- F. Materials for other surface-water management and erosion controls, including storm drain inlet protection, shall be in accordance with ODNR Rainwater and Land Development Standards.

2.05 EQUIPMENT

- A. Furnish equipment to perform work specified in this Section.

PART 3 EXECUTION

3.01 INSTALLATION

- A. Silt Fence
 - 1. Install silt fence in accordance with ODNR Rainwater and Land Development, and at the locations required by the Contractor's Surface-Water Management and Erosion Control Work Plan.
- B. Erosion Mat
 - 1. Provide erosion mat at the locations indicated on the Construction Drawings and the locations indicated on the Contractor's Surface-Water Management and Erosion Control Work Plan, and those locations resulting from Section 02930 permanent slope stabilization requirements. Begin installation of erosion mat in a specific area within 48 hours after seeding has been completed in that area. Seeding shall be as specified in Section 02930. If seeding coverage in an area is lost due to inclement weather prior to installation of the erosion mat, Contractor shall reseed the previously seeded area.
 - 2. Place erosion mat on a prepared surface that is free of deleterious vegetation, trash, ruts, and rocks.
 - 3. Overlap adjacent erosion mats in accordance with the Manufacturer's recommendations.
 - 4. Install and staple erosion mat in accordance with Manufacturer's recommendations, except staples shall be a minimum 6-inches in length.
- C. Install check dams in ditches and channels in accordance with ODNR Rainwater and Land Development Standards.
- D. Apply crusting agents in accordance with Section 02930. Areas of crusting agent application shall be approved in advance by the Construction Manager.
- E. Stabilize and vegetate disturbed areas in accordance with Section 02930.

- F. Install riprap in accordance with Section 02271 and as shown on the Construction Drawings.
- G. Construct channels, ditches, and berms as shown on the Construction Drawings and in accordance with the Contractor's Surface-Water Management and Erosion Control Work Plan. Earthwork for channels, ditches, and berms shall be in accordance with Section 02200.
- H. Install construction entrances in accordance with the Contractor's Surface-Water Management and Erosion Control Work Plan.
- I. Install additional surface-water management and erosion controls in accordance with the Contractor's Surface-Water Management and Erosion Control Work Plan.
- J. Install storm drain inlet protection in accordance with ODNR Rainwater and Land Development Standards.

3.02 ADDITIONAL REQUIREMENTS

- A. Prevent the runoff of polluting substances such as silt, clay, fuels, oils, and contaminated soils into water supplies and surface waters in accordance with the Contractor's Surface-Water Management and Erosion Control Work Plan.
- B. Remove accumulated silt and debris from behind the face of the silt fence when the silt deposits reach approximately one half the height of the fence. Replace silt fence geotextile damaged during maintenance operations. Removed silt and debris shall be placed in the OSDF constructed cells in accordance with Section 13010 or stockpiled in locations approved by the Construction Manager.

3.03 SURVEY CONTROL

- A. Survey permanent locations of surface-water management and erosion control measures in accordance with Section 02100.

3.04 MAINTENANCE

- A. Clean, maintain, repair, and replace surface-water management and erosion controls for the duration of the Contract in accordance with the Contractor's Surface-Water Management and Erosion Control Work Plan.
- B. Maintain erosion control measures and existing sedimentation basins in accordance with Part 6 of the Contract Documents.
- C. Sedimentation basins shall be cleaned of silt once per construction season.

3.05 INSPECTIONS

- A. Inspect surface-water management and erosion control measures and sedimentation basins to evaluate their effectiveness and need for maintenance. Any required repairs to the surface-water management and erosion control measures and sedimentation basins shall be initiated upon discovery, but no later than 24 hours after discovery. Inspections shall occur, at a minimum, at the following frequencies:
 - 1. weekly;
 - 2. daily after each rain event exceeding 0.5 inches; and
 - 3. at least daily during prolonged rainfall events.
- B. Records of inspections shall be kept on file on-site by Contractor and shall be submitted monthly to the Construction Manager. The records of inspection shall include the following:
 - 1. summary of the scope of the inspection;
 - 2. name of inspector;
 - 3. inspection date;
 - 4. inspection location;
 - 5. purpose of the inspection (i.e., regular weekly, following a storm, etc.);
 - 6. observations relative to performance of the surface-water management and erosion control measures;
 - 7. any necessary corrective actions; and
 - 8. corrective actions completed and their performance since the previous inspection.

3.06 CONSTRUCTION QUALITY REQUIREMENTS

- A. CQC Consultant will monitor the installation and maintenance of surface-water management and erosion control measures in accordance with this Section and the Construction Quality Assurance (CQA) Plan.

[END OF SECTION]

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SPECIFICATION COVER SHEET

SPECIFICATION SECTION: 02714 TITLE: Geotextiles

Specifications By: Signature [Signature] 17 Aug 01
(Cognizant Engineer) Date
Printed Name Michael J. Monteleone, P.E.
and Title Associate

Scope and Format
Checked By: Signature [Signature] 17 Aug 01
(Checker) Date
Printed Name Michael J. Monteleone, P.E.
and Title Associate

Detailed Requirements
Checked by: Signature [Signature] 17 Aug 01
(Checker) Date
Printed Name David K. Phillips
and Title Senior Project Engineer

Overall Review By: Signature [Signature] 20 Aug 2001
(PDP) Date
Printed Name J.F. Beech, Ph.D., P.E.
and Title Principal

Approved by: Signature [Signature] 20 Aug 2001
(DTL) Date
Printed Name J.F. Beech, Ph.D., P.E.
and Title Principal

Record of Revision (Number and initial all revisions)

Rev. No.	Reason	Date	By	Checked	Approval
0	Certified for Construction	20 August 01	MSR	DKP	ATB
1	Revisions from Phase III DCNs	13 Mar 02	DKP	DKP	ATB

SECTION 02714

GEOTEXTILES

PART 1 GENERAL

1.01 SCOPE

- A. This Section includes materials and installation for geotextiles.
- B. Quantity of geotextile materials to be furnished by Fluor Fernald, Inc. will be as specified in Part 6 of the Contract Documents. Additional required geotextile materials, shall be furnished by the Contractor.

1.02 RELATED SECTIONS AND PLANS

- A. Section 02200 - Earthwork
- B. Section 02215 - Trenching and Backfilling
- C. Section 02230 - Road Construction
- D. Section 02271 - Riprap
- E. Construction Quality Assurance (CQA)
- F. Part 6 - Statement of Work
- G. Part 8 - Environmental Health & Safety/Training Requirements
- H. Part 9 - Quality Assurance Requirements

1.03 REFERENCES

- A. Latest version of American Society for Testing and Materials (ASTM) Standards:
 - 1. ASTM D 4355. Standard Test Method for Deterioration of Geotextiles from Exposure to Ultraviolet Light and Water (Xenon-ARC type apparatus).
 - 2. ASTM D 4491 Standard Test Method for Water Permeability of Geotextiles by Permittivity.
 - 3. ASTM D 4533. Standard Test Method for Trapezoid Tearing Strength of Geotextiles.
 - 4. ASTM D 4632. Standard Test Method for Breaking Load and Elongation of Geotextiles (Grab Method).

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5. ASTM D 4751. Standard Test Method for Determining Apparent Opening Size of a Geotextile.
6. ASTM D 4833. Standard Test Method for Index Puncture Resistance of Geotextiles, Geomembranes, and Related Products.
7. ASTM D 4873. Standard Guide for Identification, Storage, and Handling of Geotextiles.
8. ASTM D 5261. Standard Test Method for Measuring Mass Per Unit Area of Geotextiles.
9. ASTM D 5493. Standard Test Method for Permittivity of Geotextiles Under Load.
10. ASTM D 6241. Standard Test Method for the Static Puncture Strength of Geotextiles and Geotextile Related Products Using a 50-mm Probe.

B. Federal Standard No. 751a - Stitches, Seams, and Stitching.

1.04 SUBMITTALS

- A. Submit to the Construction Manager a letter of acceptance for the quantity of geotextile materials furnished by Fluor Fernald, Inc. Quantity of geotextile materials stored on site shall be inspected, inventoried, and accepted within 30 calendar days of Notice to Proceed.
- B. For geotextiles furnished by Contractor, submit the following to Construction Manager for review within 30 calendar days from Notice to Proceed:
 1. product name;
 2. geotextile manufacturing capabilities, including;
 - a. daily production capacity available for this Contract; and
 - b. manufacturing quality control procedures;
 3. certification of minimum average roll values 95 percent lower confidence limits and the corresponding test procedures for all geotextile properties listed in Tables 02714-1 to 02714-5;
 4. projected geotextile delivery dates; and
 5. recommended long-term storage requirements and limitations.
- C. For geotextiles furnished by Contractor, submit to Construction Manager for review at least 14 calendar days prior to transporting geotextile to the site, manufacturing quality control certificates signed by the quality control manager applicable to each roll of geotextile as specified in this Section. The submittal shall include a list of roll numbers to be shipped indicating which rolls were sampled and tested. The certificates shall state that the geotextiles are continuously inspected and are needle-free. The quality control certificates shall also include:
 1. lot, roll numbers, and other identification;

2. sampling procedures; and
 3. results of quality control tests, including descriptions of test methods used (the Manufacturer quality control tests to be performed are specified in this Section).
- D. Provide list of equipment, description of installation methods storage methods in accordance with manufacturer's recommendation, and other required information related to the installation of geotextile in the Earthwork Work Plan specified in Section 02200.

1.05 HEALTH AND SAFETY REQUIREMENTS

- A. Environmental health & safety/training requirements shall be in accordance with Part 8 of the Contract Documents.

1.06 CONTRACTOR'S QUALITY ASSURANCE

- A. Contractor's quality assurance requirements shall be in accordance with Part 9 of the Contract Documents.

PART 2 PRODUCTS

2.01 GEOTEXTILE

- A. Geotextile materials furnished by Fluor Fernald, Inc. will meet the following requirements:
1. minimum average roll values with 95 percent lower confidence limits meeting or exceeding the required property values specified in Tables 02714-1 for geotextile filters and sacrificial geotextile filters, 02714-2 for geotextile cushion in final cover system, 02714-3 for geotextile cushion in liner system, 02714-4 for supplemental geotextile cushion in liner and scaraficial geotextile cushion in the final cover systems, and 02714-5 for geotextile separator; and
 2. manufactured from first quality polymers, with not more than 20 percent reclaimed polymer used in production.
- B. Geotextiles furnished by Contractor shall meet or exceed the required property values specified in Tables 02714-1 through 02714-5. Geotextiles shall be manufactured from first quality polymers with not more than 20 percent reclaimed polymer used in production.
- C. Furnish polymeric threads for stitching that are ultra-violet (UV) light stabilized to at least the same requirements as the geotextile to be sewn. Threads shall be polyester or polypropylene threads that have a minimum size of 2,000 denier.

2.02 MANUFACTURING QUALITY CONTROL

- A. For geotextile furnished by Contractor, sample and test the geotextile to demonstrate that the material conforms to the requirements of this Section. Do not supply any geotextile roll that does not comply with the manufacturing quality control requirements.

1. Perform manufacturing quality control tests to demonstrate that properties conform to the values specified in Tables 02714-1 to 02714-5. Perform the following manufacturing quality control tests at a maximum interval of one test for each 50,000 square feet manufactured. All tested rolls of material used to certify compliance shall be delivered to the site. Test data for rolls not delivered to the site will not be accepted.

TestProcedure

Mass per unit area	ASTM D 5261
Grab strength	ASTM D 4632
Tear strength	ASTM D 4533
Puncture strength	ASTM D 4833 or ASTM D 6241

2. Perform additional manufacturing quality control tests on geotextile filter properties only, at a maximum interval of one test for each 100,000 square feet manufactured to demonstrate that its apparent opening size (ASTM D 4751) and permittivity (ASTM D 4491 or ASTM D 5493) conform to the values specified in Table 02714-1. All tested rolls of material used to certify compliance shall be delivered to the site. Test data for rolls not delivered to the site will not be accepted.

- B. For geotextile furnished by Contractor, if a geotextile sample fails to meet the quality control requirements of this Section, sample and test rolls manufactured at the same time and in the same lot as the failing roll. Continue to sample and test the rolls until the extent of the failing rolls are bracketed by passing rolls. Do not supply failing rolls.

2.03 PACKAGING

- A. Geotextiles rolls will be wrapped in relatively impermeable and opaque protective covers.
- B. Covers which become torn or damaged shall be repaired by the Contractor with similar materials.
- C. Geotextile rolls will be marked or tagged in accordance with ASTM D 4873 with the following information:

1. Manufacturer's name;
 2. product identification;
 3. lot or batch number;
 4. roll number; and
 5. roll dimensions.
- D. Geotextile rolls not labeled in accordance with this Section or on which labels are illegible shall be rejected and replaced. The Contractor shall notify the Construction Manager of any rolls not labeled in accordance with the Section.
- E. The minimum size of each lot for geotextile furnished by the Contractor and delivered to the Site will be 100,000 ft².

2.04 SHIPPING

- A. Geotextiles furnished by Contractor, shall not be shipped prior to final review and confirmation of compliance of Manufacturer's quality control submittals specified in this Section and conformance testing specified in the CQA Plan and by the Consultant

2.05 ACCEPTANCE, HANDLING, AND STORAGE

- A. Upon delivery to the project site, Contractor shall inspect and inventory the geotextile materials and the manner in which they are stored. Contractor shall also inspect geotextile material stockpiled at the OSDF. Contractor shall provide to the Construction Manager with a written letter of acceptance within 30 calendar days if material is acceptable for installation. Contractor shall also notify the Construction Manager in writing within 30 calendar days of any geotextile material that is not acceptable for installation.
- B. Protection and preservation of geotextile material shall include, but not be limited to:
1. protection from sunlight, moisture, excessive heat or cold, puncture, mud, dirt, and dust or other damaging conditions; follow geotextile Manufacturer recommendations for handling and storage; Manufacturer recommendations will be provided by the Construction Manager for geotextile furnished by Fluor Fernald, Inc.; and
 2. storage of rolls on pallets, or other elevated structures; do not store rolls directly on the ground.
- C. Contractor shall unload, handle and store geotextile material furnished by Fluor Fernald, Inc. and by Contractor. Handling shall be performed such that damage to geotextile materials does not occur.

2.06 EQUIPMENT

- A. Furnish equipment for acceptance, handling, storage, and installation of geotextile.

PART 3 EXECUTION

3.01 PLACEMENT

- A. Do not commence geotextile installation until the CQC Consultant completes performance testing and confirmation of compliance of underlying layers, including acceptance of Contractor's survey results for underlying layers.
- B. Handle geotextiles so as to ensure they are not damaged.
- C. Take precautions to prevent damage to underlying layers.
- D. After unwrapping the geotextiles from their opaque covers, do not leave them exposed for a period in excess of 10 calendar days or for the Manufacturer's written recommended exposure period.
- E. If white colored geotextiles are used, take appropriate safety precautions against "snowblindness" of personnel.
- F. Take care not to entrap stones, excessive dust, or moisture below or in the geotextiles.
- G. Examine the geotextile surface after installation to ensure that no potentially harmful foreign objects are present. Remove any such objects and replace any damaged geotextiles.

3.02 SEAMS AND OVERLAPS

- A. Continuously overlap a minimum of 6 inches and sew geotextile filters, cushions, and supplemental geotextile cushions using a "single prayer" seam. Sew seams using Stitch Type 401 as per Federal Standard No. 751a. Spot sewing will not be allowed.
- B. Do not install horizontal seams on slopes that are steeper than 10 horizontal to 1 vertical (10H:1V). Seams shall be along, not across, the slopes.
- C. Overlap geotextile separator a minimum of 12 inches and spot sew at intervals to ensure that the overlap is maintained.
- D. Overlap geotextile filter used for riprap construction in Section 02271 a minimum of 12 inches. No seaming is required.

3.03 REPAIR

- A. Repair holes or tears in the geotextiles using a patch made from the same geotextile material. Extend geotextile patches a minimum of 1 foot beyond the damaged area. Sew geotextile patches into place no closer than 1 inch from panel edge. Should tear exceed 50 percent of the width of the panel, cut across the entire width of the panel and seam as an end seam. For slope areas steeper than 10H:1V, tears exceeding 50 percent of the width of the panel shall be removed and replaced.
- B. Remove any soil or other material that may have penetrated the torn geotextiles.

3.04 CREST ANCHORAGE SYSTEM

- A. Install the geotextile along with the other geosynthetic layers in the anchor trench and wedge at the crest of the slope as shown on the Construction Drawings. Temporarily anchor the geosynthetic layers using sandbags or other means until the commencement of trench backfilling. Do not place geotextiles in anchor trench if standing water is present.
- B. Do not entrap soil, sand bags, excessive moisture, or other materials below or between the geosynthetic layers in the anchor trench.
- C. Backfill the anchor trench with compacted clay liner once all the geosynthetic layers are installed in the anchor trench. Backfill to the limits shown on the Construction Drawings. Compact backfill in accordance with Section 02215.
- D. Do not damage exposed geosynthetic layer when backfilling the anchor trench.
- E. Do not place granular drainage material for the leak detection system or leachate collection system on the side slopes until after the anchor trenches are completely backfilled unless authorized in writing by the Construction Manager.

3.05 PLACEMENT OF SOIL AND AGGREGATE MATERIALS

- A. Place soil materials on top of geotextiles in such a manner as to ensure that:
 - 1. the geotextiles and the underlying materials are not damaged; and
 - 2. slippage does not occur between the geotextile and the underlying layers during placement.
- B. Spread soil on top of the geotextile to cause the soil to cascade onto the geotextile rather than be shoved across the geotextile.
- C. For geotextile cushions overlying the geomembrane, do not place granular drainage material at ambient temperatures below 40 degrees Fahrenheit (°F) or above 104°F.

For placement of granular drainage material below 40°F and above 104°F, Contractor shall submit placement methods to the Construction Manager for review and approval.

- D. Do not drive equipment directly on the geotextile. Only use equipment above a geotextile cushion overlying a geomembrane that meets the following ground pressure requirements:

Maximum Allowable Equipment Ground Pressure (pounds per square inch)	Minimum Thickness of Overlying Fill or Aggregate Layer (inches)
less than 5	12 (see note 1)
less than 10	18
less than 20	24
greater than 20	36

Note 1: Minimum thickness of first loose lift of the clay liner material for the protective clay layer shall be a 10-inch \pm 1-inch.

- E. Place aggregate over geotextile separator as shown on the Construction Drawings prior to trafficking in accordance with Section 02230.
- F. Place soil over geotextile filters as shown on the Construction Drawings prior to trafficking.

3.06 CONSTRUCTION QUALITY REQUIREMENTS

- A. CQC Consultant will perform conformance testing on the geotextile materials furnished by Contractor to establish compliance with this Section. Conformance testing and minimum frequencies shall be in accordance with the Construction Quality Assurance (CQA) Plan.
- B. CQC Consultant will monitor the geotextile installation in accordance with this Section and CQA Plan.

TABLE 02714-1
REQUIRED PROPERTY VALUES FOR GEOTEXTILE FILTER AND
SACRIFICIAL GEOTEXTILE FILTER

PROPERTIES	QUALIFIER	UNITS ⁽⁴⁾	SPECIFIED ⁽³⁾ PROPERTY VALUES	TEST METHOD
<u>Identification Requirements</u>				
Type	(-)	(-)	Nonwoven needlepunched	(-)
Polymer composition	minimum	%	95 polypropylene or polyester by weight	(-)
Mass per unit area	minimum	oz/yd ²	7	ASTM D 5261
<u>Filter Requirements</u>				
Apparent opening size (O ₉₅)	maximum	mm	0.212	ASTM D 4751
Permittivity	minimum	sec ⁻¹	0.5	ASTM D 4491 or ASTM D 5493
<u>Mechanical Requirements</u>				
Grab strength	minimum	lb	180	ASTM D 4632 ⁽¹⁾
Trapezoidal tear strength	minimum	lb	75	ASTM D 4533 ⁽²⁾
Puncture strength	minimum	lb	75	ASTM D 4833
Static puncture strength	minimum	lb	450	ASTM D 6241
<u>Durability Requirements</u>				
Ultraviolet Resistance	minimum	%	70	ASTM D 4355

Notes:

- (1) Minimum of values measured in machine and cross machine directions with 1 by 2 inch clamp on Constant Rate of Extension (CRE) machine.
- (2) Minimum value measured in machine and cross machine direction.
- (3) All values represent minimum average roll values.
- (4) mm = millimeter
% = percent
oz/yd² = ounce per square yard
sec = second
lb = pound
psi = pound per square inch

TABLE 02714-2
REQUIRED PROPERTY VALUES FOR GEOTEXTILE CUSHION
IN FINAL COVER SYSTEM

PROPERTIES	QUALIFIER	UNITS ⁽⁴⁾	SPECIFIED ⁽¹⁾ PROPERTY VALUES	TEST METHOD
<u>Identification Requirements</u>				
Type	(-)	(-)	Nonwoven needlepunched	(-)
Polymer composition	minimum	%	95 polypropylene or polyester by weight	(-)
Mass per unit area	minimum	oz/yd ²	8	ASTM D 5261
<u>Mechanical Requirements</u>				
Grab strength	minimum	lb	200	ASTM D 4632 ⁽¹⁾
Tear strength	minimum	lb	75	ASTM D 4533 ⁽²⁾
Puncture strength	minimum	lb	90	ASTM D 4833
Static puncture strength	minimum	lb	500	ASTM D 6241
<u>Durability Requirements</u>				
Ultraviolet Resistance	minimum	%	70	ASTM D 4355

Notes:

- (1) Minimum of values measured in machine and cross machine directions with 1 by 2 inch clamp on Constant Rate of Extension (CRE) machine.
- (2) Minimum value measured in machine and cross machine direction.
- (3) All values represent minimum average roll values.
- (4) mm = millimeter
% = percent
oz/yd² = ounce per square yard
sec = second
lb = pound
psi = pound per square inch

TABLE 02714-3
REQUIRED PROPERTY VALUES FOR GEOTEXTILE CUSHION
IN LINER SYSTEM

PROPERTIES	QUALIFIER	UNITS ⁽⁴⁾	SPECIFIED ⁽³⁾ PROPERTY VALUES	TEST METHOD
<u>Identification Requirements</u>				
Type	(-)	(-)	Nonwoven needlepunched	(-)
Polymer composition	minimum	%	95 polypropylene or polyester by weight	(-)
Mass per unit area	minimum	oz/yd ²	10	ASTM D 5261
<u>Mechanical Requirements</u>				
Grab strength	minimum	lb	225	ASTM D 4632 ⁽¹⁾
Tear strength	minimum	lb	90	ASTM D 4533 ⁽²⁾
Puncture strength	minimum	lb	120	ASTM D 4833
Static puncture strength	minimum	lb	675	ASTM D 6241
<u>Durability Requirements</u>				
Ultraviolet Resistance	minimum	%	70	ASTM D 4355

Notes:

- (1) Minimum of values measured in machine and cross machine directions with 1 by 2 inch clamp on Constant Rate of Extension (CRE) machine.
- (2) Minimum value measured in machine and cross machine direction.
- (3) All values represent minimum average roll values.
- (4) mm = millimeter
% = percent
oz/yd² = ounce per square yard
sec = second
lb = pound
psi = pound per square inch

TABLE 02714-4

**REQUIRED PROPERTY VALUES FOR SUPPLEMENTAL AND SACRIFICIAL
GEOTEXTILE CUSHION IN LINER SYSTEM
AND FINAL COVER SYSTEM**

PROPERTIES	QUALIFIER	UNITS ⁽⁴⁾	SPECIFIED ⁽³⁾ PROPERTY VALUES	TEST METHOD
<u>Identification Requirements</u>				
Type	(-)	(-)	Nonwoven needlepunched	(-)
Polymer composition	minimum	%	95 polypropylene or polyester by weight	(-)
Mass per unit area	minimum	oz/yd ²	16	ASTM D 5261
<u>Mechanical Requirements</u>				
Grab strength	minimum	lb	350	ASTM D 4632 ⁽¹⁾
Tear strength	minimum	lb	120	ASTM D 4533 ⁽²⁾
Puncture strength	minimum	lb	180	ASTM D 4833
Static puncture strength	minimum	lb	1,275	ASTM D 6241
<u>Durability Requirements</u>				
Ultraviolet Resistance	minimum	%	70	ASTM D 4355

Notes:

- (1) Minimum of values measured in machine and cross machine directions with 1 by 2 inch clamp on Constant Rate of Extension (CRE) machine.
- (2) Minimum value measured in machine and cross machine direction.
- (3) All values represent minimum average roll values.
- (4) mm = millimeter
% = percent
oz/yd² = ounce per square yard
sec = second
lb = pound
psi = pound per square inch

TABLE 02714-5

REQUIRED PROPERTY VALUES FOR GEOTEXTILE SEPARATOR

PROPERTIES	QUALIFIER	UNITS ⁽⁴⁾	SPECIFIED ⁽³⁾ PROPERTY VALUES	TEST METHOD
<u>Identification Requirements</u>				
Type	(-)	(-)	Nonwoven needlepunched	(-)
Polymer composition	minimum	%	95 polypropylene or polyester by weight	(-)
Mass per unit area	minimum	oz/yd ²	6	ASIM D 5261
<u>Mechanical Requirements</u>				
Grab strength	minimum	lb	180	ASTM D 4632 ⁽¹⁾
Tear strength	minimum	lb	75	ASTM D 4533 ⁽²⁾
Puncture strength	minimum	lb	75	ASTM D 4833
Static puncture strength	minimum	lb	450	ASTM D 6241
<u>Durability Requirements</u>				
Ultraviolet Resistance	minimum	%	70	ASTM D 4355

Notes:

- (1) Minimum of values measured in machine and cross machine directions with 1 by 2 inch clamp on Constant Rate of Extension (CRE) machine.
- (2) Minimum value measured in machine and cross machine direction.
- (3) All values represent minimum average roll values.
- (4) mm = millimeter
% = percent
oz/yd² = ounce per square yard
sec = second
lb = pound
psi = pound per square inch

[END OF SECTION]

SPECIFICATION COVER SHEET

SPECIFICATION SECTION: 02930 TITLE: Vegetation

Specifications By: Signature [Signature] 17 Aug 01
(Cognizant Engineer) Date
Printed Name Michael J. Monteleone, P.E.
and Title Associate

Scope and Format
Checked By: Signature [Signature] 17 Aug 01
(Checker) Date
Printed Name Michael J. Monteleone, P.E.
and Title Associate

Detailed Requirements
Checked by: Signature [Signature] 17 Aug 01
(Checker) Date
Printed Name David K. Phillips
and Title Senior Project Engineer

Overall Review By: Signature [Signature] 20 Aug 2001
(PDP) Date
Printed Name J.F. Beech, Ph.D., P.E.
and Title Principal

Approved by: Signature [Signature] 20 Aug 2001
(DTL) Date
Printed Name J.F. Beech, Ph.D., P.E.
and Title Principal

Record of Revision (Number and initial all revisions)

Rev. No.	Reason	Date	By	Checked	Approval
0	Certified for Construction	20 August 01	MSW	DIC	TFB
1	Revisions from Phase III DCNs	13 March 02	DIC	DIC	TFB

SECTION 02930**VEGETATION****PART 1 GENERAL****1.01 SCOPE**

- A. This Section includes soil stabilization, which includes application of crusting agent and establishing vegetation by seeding. The work in this Section includes, but is not limited to; soil preparation, interim vegetation, permanent vegetation, application of fertilizer, application of mulches, and application of crusting agent.

1.02 RELATED SECTIONS AND PLANS

- A. Section 02200 - Earthwork
- B. Section 02270 - Surface-Water Management and Erosion Control
- C. Part 6 - Statement of Work
- D. Part 8 - Environmental Health & Safety/Training Requirements
- E. Part 9 - Quality Assurance Requirements

1.03 REFERENCES

- A. Latest version of Ohio Department of Natural Resources (ODNR) Rainwater and Land Development Standards (ODNR Rainwater and Land Development Standards).
- B. "Identification and Listing of Hazardous Waste", Title 40, Code of Federal Regulations (CFR), Part 261, Subpart E.C.
- C. "Federal Hazardous Material Transportation Law", U.S. Department of Transportation [U.S. DOT, 1994].

1.04 SUBMITTALS

- A. Submit the following to the Construction Manager for review within 15 calendar days from Notice to Proceed:
 - 1. proposed mixes and application rates for seed, mulch, fertilizers, and crusting agents;

2. Manufacturer's product data and recommended methods of application for seed, mulches, fertilizer, and crusting agents;
 3. product data for fertilizer shall also include chemical analysis including uranium analysis to assure there is no resultant or derived uranium from fertilizer use, unless waived by Construction Manager;
 4. material safety data sheet (MSDS) for fertilizer, mulch binder and crusting agent; and
 5. inoculant information for the permanent seed mixes.
- B. Submit the following to the Construction Manager for review within 30 calendar days before seeding:
1. certificate stating seed mixture, guaranteed percentages of purity, weed content, germination of seed, name of seller, test date for the seed, and the net weight and date of shipment;
 2. Manufacturer's certificate stating the available nutrients contained in the proposed fertilizer;
 3. Manufacturer's certificate stating that the fiber matrix (wood fibers) meets the requirements of this Section;
 4. Manufacturer's certificate stating the mulch binder meets the requirements of this Section;
 5. Manufacturer's certificate stating the crusting agent meets the requirements of this Section; and
 6. documentation of the straw to be used for mulch; this documentation shall verify that the straw is weed free in accordance with the requirements of this Section.
- C. Submit to the Construction Manager for review within 10 calendar days before seeding a plan showing seeding area and a written statement of application rate of seed mix and/or associated materials (i.e., fertilizer, mulch, and mulch binder). Choice of seeding type shall follow the site seeding requirements and as approved by the Construction Manager.
- D. Provide a list of equipment, description of construction methods, and other required information for vegetation and application of crusting agent in the Contractor's Earthwork Work Plan specified in Section 02200.

1.05 HEALTH AND SAFETY REQUIREMENTS

- A. Environmental health & safety/training requirements shall be in accordance with Part 8 of the Contract Documents.

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1.06 CONTRACTOR'S QUALITY ASSURANCE

- A. Contractor's quality assurance requirements shall be in accordance with Part 9 of the Contract Documents.

PART 2 PRODUCTS

2.01 MATERIALS

- A. Furnish seed labeled in accordance with U.S. Department of Agriculture (USDA) Rules and Regulations under the Federal Seed Act and applicable State seed laws. Furnish seed in sealed bags or containers bearing the date of expiration. Do not use seed after its date of expiration. Each variety of seed shall have a purity of not less than 90 percent by weight, a percentage of germination not less than 80 percent by weight, and a weed to seed content of not more than 0.75 percent by weight and contain no noxious weeds. Furnish seed mixtures having seed proportioned by weight in accordance with Tables 02930-1A, 02930-1B, 02930-1C and 02930-2. Areas requiring permanent seeding during the summer months (June 15 – September 20), excluding the OSDF Cell Final Cover, shall be seeded with 30 lbs/acre of ReGreen as specified in this Section. An alternative to ReGreen, and the only acceptable alternative for summer seeding of the OSDF Cell Final Cover, is stabilizing with a crusting agent as specified in this Section. Stabilization performed during the summer shall be followed by fall application of the appropriate permanent seed mix.
- B. Permanent seed mixes shall be treated with fungal (Mycorrhizae) inoculant and bacterial (Rhizobium) inoculants. The specified legumes must be inoculated with the appropriate Rhizobial strains.
- C. Furnish mulch meeting the following requirements:
 - 1. Mulch shall be straw or wood cellulose fiber, free of clay, stone, foreign substances, and free of weeds.
 - 2. Straw should not contain sticks larger than ¼-inch diameter or other materials that may prevent matting down during application. Use straw that is free from mold and other objectionable material for placing with mulch blower equipment or other equipment as approved by the Construction Manager. Straw shall be generally 6 inches or more in length.
 - 3. Straw shall be:
 - a. weed free straw from the Minnesota Crop Improvement Association certified weed free straw vendors;
 - b. straw that has been inspected and determined to be weed free by Central Ohio Seed Testing;

- c. native prairie grass mulch; or
 - d. equivalent substitute as approved by the Construction Manager.
4. Mulch applied by hydrospraying shall be a bonded fiber matrix containing wood fibers held together with a hydrocolloid-based binder, which upon drying becomes insoluble and non-dispersible. Mulch shall be comprised of 39 parts wood fiber to one part binder by weight. The fibers shall be composed of 100 percent wood or wood by-products and shall be 100 percent biodegradable. Use a bonded fiber matrix containing a green dye that will provide for easy visual inspection for uniformity of slurry spread. The bonded fiber matrix, including dye, shall contain no growth or germination inhibiting properties. The wood cellulose fiber shall be manufactured in such a manner that, after addition and agitation in slurry tanks with water, the fibers in the material become uniformly suspended to form a homogeneous material. When sprayed on the ground, the material shall allow absorption and percolation of moisture. The wood cellulose fiber shall meet the following requirements:

<u>Item</u>	<u>Specification Limit</u>
Particle Length	0.8 inch (maximum)
Particle Thickness	0.047 inch (maximum)
Ph	4.0 to 8.5
Ash Content	1.6% (maximum)
Water Holding Capacity (based on fiber dry weight)	500% (minimum)
Moisture Content	12% \pm 4% (by weight)

- D. Mulch binder agent shall be as approved by the Construction Manager and shall meet the following requirements:
- 1. The mulch binder shall be hydrocolloid base (guar gum) and shall not dissolve or disperse upon rewetting.
 - 2. The mulch binder shall not have hazardous characteristics of ignitability, corrosivity, reactivity, or toxicity as defined in 40 CFR Part 261, Subpart C, for a hazardous waste in either its pre-applied or cured states.
 - 3. The mulch binder shall have a flash point greater than 200°F. The mulch binder shall be neither a flammable nor combustible liquid per United States (US) Department of Transportation definition [U. S. DOT, 1994]. The mulch binder must not be susceptible to significant deterioration from exposure to the elements, including sunlight.
 - 4. The mulch binder shall be provided in concentrated solution and prepared so that it will not change in transportation or storage.

- E. The crusting agent shall be as approved by the Construction Manager and shall meet the following criteria:
1. pine sap emulsion comprised of a 100 percent organic emulsion produced from naturally occurring resins (pine sap); or an approved equal;
 2. not comprised of chloride, lignosulfonate, petroleum, or asphaltic-type emulsions;
 3. provide dust suppression and surface stability for exposed soils, both disturbed and undisturbed soils, and exposed coal fired ash (fly ash);
 4. compatible with application via a hydro seeder, and must not require intense cleaning of equipment after application;
 5. non-tracking (i.e., will not stick to boots or tires) once cured;
 6. not have hazardous characteristics of ignitability, corrosivity, reactivity, or toxicity as defined in 40 CFR Part 261, Subpart C, for a hazardous waste in either its pre-applied or cured states;
 7. have a flash point greater than 200 °F;
 8. be neither a flammable nor combustible liquid per DOT definition; and
 9. not be susceptible to significant deterioration from exposure to the elements, including sunlight.
- F. Erosion mat shall be in accordance with Section 02270.
- G. Fertilizer:
1. Furnish commercial grade fertilizer, uniform in composition that meets the requirements of all State and Federal regulations and standards of the Association of Agricultural Chemists.
 2. Fertilizer shall be slow release complete fertilizer.
 3. Two types of fertilizer mixes shall be used. Fertilizer for application within the former production area shall be 34-0-10; other fertilizers may be approved by the Construction Manager for the former production area, but they must not contain phosphorous. Fertilizer for other areas shall be 22-5-10. Other fertilizers may be approved by the Construction Manager for areas outside the former production area provided the fertilizer mix does not contain more than 6% phosphorous. Fertilizers shall contain not less than 1 percent added sulfur and not more than 8 percent added iron, or an approved equal.
 4. Fertilizer must have MSDS submitted in accordance with this Section.
 5. Fertilizer shall be used for interim seeding only.
- H. Construction water shall be obtained from the on-site water source shown on the Construction Drawings.

2.02 EQUIPMENT

- A. Provide equipment of size and type to perform work specified in this Section.

PART 3 EXECUTION

3.01 DELIVERY, STORAGE, AND HANDLING

- A. Deliver containerized materials in uniform packages bearing the name of the manufacturer, the net weight and a statement of content. Deliver containerized materials to the site in original, properly labeled, unopened, clean containers each showing the manufacturer's guaranteed analysis conforming to applicable regulations and standards.
- B. Store materials in a dry area in a manner to prevent physical damage.

3.02 GENERAL

- A. Stabilization of disturbed areas by vegetation or by use of a crusting agent shall be performed at completion of excavation and stockpiles or within 7 calendar days of knowing a disturbed area will be idle for more than 45 calendar days, whichever is sooner.
- B. Crusting agents may be used as temporary measures prior to placement of interim vegetation after approval for the area by the Construction Manager.
- C. Interim vegetation, as specified in this Section, is required for all areas except OSDF final cover system and soil stockpiles, which are scheduled to be disturbed in future. Fertilizer shall be used for interim vegetation as specified in this Section.
- D. Permanent vegetation, as specified in this Section, is required for OSDF final cover system. No fertilizer shall be used with permanent vegetation as specified in this Section.
- E. Disturbed areas which are scheduled to be significantly disturbed after initial stabilization and/or need effective erosion control immediately, are to be stabilized with the interim seed mix rate specified in this Section. Disturbed areas which are not scheduled to be significantly disturbed again are to be stabilized with the permanent seed mix rate specified in this Section. Soil piles, which require effective erosion control immediately, are to be stabilized with the interim seed mix rate or a crusting agent as specified in this Section.
- F. Use an erosion mat as specified in Section 02270 at locations shown on the Construction Drawings after application of seed mixture.
- G. Area(s) to be seeded shall be generally free of debris, rock, root material, and other objects that may impede soil preparation and seeding activities. Perform soil

preparation by tilling/cultivating, to a depth of approximately 2 inches, to eliminate uneven areas and low spots. Maintain lines, levels and contours.

- H. Repeat cultivation in areas where equipment used for hauling and spreading has compacted the area(s) to be seeded.

3.03 APPLICATION

- A. The seeding season, for interim vegetation specified in this Section, is year round. However, if seeding is contemplated during the winter months of December through March, then field conditions should be assessed for ability to provide soil to seed contact. If field conditions do not support the ability to provide soil to seed contact then the area shall be stabilized with a crusting agent followed by seeding during conditions conducive to adequate soil to seed contact.
- B. The permanent seeding in wet and dry areas and the cell final cover shall be performed in the Spring Season between April 1 and June 15 and/or the Fall Season between September 20 and November 30, unless otherwise approved by the Construction Manager.
- C. Apply fertilizer, seed, and mulch to disturbed areas and areas excavated and graded under this Contract requiring seeding unless otherwise directed by the Construction Manager. Apply mulch within 24 hours of seeding; do not seed areas in excess of that which can be mulched within 24 hours. Winter application of seed and related materials are subject to adjustment as directed by the Construction Manager.
- D. Apply seed using either the drilling, broadcasting, or hydroseeding method, as described below:
 - 1. Seed drilling method:
 - a. This method shall be used for applying the permanent seed mix in accessible areas unless otherwise approved by the Construction Manager. The method may also be used for interim vegetation.
 - b. Prepare area to be seeded by loosening the soil to a minimum depth of 3 inches.
 - c. Apply commercial grade, slow release complete fertilizer, for interim vegetation only, at a rate of 150 lbs/acre at the time of preparing the seedbed for seeding.
 - d. Install seed with a seed drill to obtain a final planting depth of $\frac{1}{4}$ to $\frac{1}{2}$ inch using the seed rates indicated in Tables 02930-1A, 02930-1B, 02930-1C and 02930-2. All seed drilling should be done perpendicular to the direction of surface-water flow.

2. Broadcast Seeding Method:

- a. This method may be used for interim vegetation, and can be performed with the use of mechanical "cyclone" seeders, by hand seeding or by any other method which scatters seed over the soil surface.
- b. This method may also be used for permanent seeding in areas that are not accessible by the seed drill method or areas where seed drilling cannot be performed perpendicular to the direction of the surface-water flow.
- c. If Broadcast Method is used to apply permanent seed mix in sloped areas (3H:1V slope or steeper), seeding application rates in Tables 02930-1A and 02930-1B should be doubled.
- d. Prepare the area to be seeded by loosening the soil to a minimum depth of 3 inches. This is critical to allow seeds to filter into the soil to avoid washout from runoff.
- e. Apply commercial grade, slow release complete fertilizer, for interim vegetation only, at a rate of 150 lbs/acre at the time of preparing the seedbed for seeding.
- f. Install seed by broadcasting evenly over the entire site using the seed rates indicated in this Section.
- g. After application of seed, perform the following prior to placement of erosion mat.
 - i. For areas receiving seed mix for cell final cover permanent vegetation (Table 02930-1C), roll seeded area with a 200 to 600 pound drum roller after seeding. If surface is not accessible for the drum roller after seeding, apply sprayed mulch at 1500 pounds per acre minimum and 100 percent continuous coverage. Mix the mulch with water at a ratio of 50 pounds of mulch per 100 gallons of water.
 - ii. For areas receiving other seed mixes (Tables 02930-1A, 02930-1B or 02930-2), rake seeded area after seeding
- h. Mulch and disc-anchor using weed free mulch at a rate of 2.0 tons per acre. Spread straw mulch, either by hand or by blowing method, at the rate of 2 air-dried tons per acre. During June through September, increase straw mulch application rate to 3 air-dried tons per acre. Application of straw mulch by the blowing method is exempt from the dust control requirements specified in Part 6 of the Contract Documents.3.

Hydroseeding Method:

- a. This method may be used for interim vegetation only. Hydroseeding shall be a two-step process. The seed shall be applied first, followed by a separate application of the mulch. This is to ensure soil to seed contact.
- b. The mixture tank shall be cleaned prior to use to ensure remnant seed is not introduced to the proposed seed mixture.

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- c. Prepare area to be seeded by loosening the soil to a minimum depth of 3 inches. This is critical to allow seeds to filter into the soil to avoid washout from runoff.
 - d. Apply commercial grade, slow release complete fertilizer, for interim vegetation only, at a rate of 150 lbs/acre. The fertilizer is to be mixed and applied with the mulch.
 - e. Install seed by hydroseeding evenly over the entire area using the seed rates indicated in Table 02930-2. Use a fan-type nozzle with approximately 500 gallons of water per acre to ensure even distribution.
 - f. Rake the area where accessible following seeding.
 - g. Apply sprayed mulch at a net dry weight of 2,000 pounds per acre minimum and 100 percent continuous coverage. Mix the mulch with water at a ratio of 50 pounds of mulch per 100 gallons of water.
- E. Application of Crusting Agent:
- 1. Apply crusting agent in accordance with manufacturer's directions.
 - 2. Unless otherwise specified by the manufacturer, dilute concentrated pine sap emulsion to ratio of 4 parts water to 1 part concentrate. Apply diluted pine sap emulsion at a rate of 2,500 gallons per acre.

3.04 MAINTENANCE

- A. Maintain the vegetated areas in satisfactory condition until acceptance of the vegetation by the Construction Manager. Maintenance of the vegetated areas includes repairing eroded areas, revegetating when necessary, watering, and mowing (if applicable). A satisfactory condition of vegetated area is defined as follows:
- 1. an area shall have a predominant stand of the seeded vegetation;
 - 2. within 3 weeks, germination must occur over 90 percent of the area with no single bare area greater than 3 square feet; and
 - 3. within 3 months, 90 percent of the area must be covered with mature vegetation.
- B. The above timeframes for germination and coverage requirements are to be delayed during the dormant season between November 1 and March 15 application of the seed. The performance criteria shall be measured at the beginning of the growing season (April 1) for seed applied during the previous dormant season.
- C. Areas that fail to meet these requirements shall be repaired or reseeded as necessary to produce an acceptable stand of vegetation, as specified in this Section.
- D. The acceptance inspection will be performed by the Construction Manager who will determine whether repair of vegetated areas or revegetation is required.

- E. Maintain areas with a crusting agent to ensure proper erosion control. The crusting agent shall be reapplied to eroded and bare areas as necessary.

3.05 WARRANTY

- A. Vegetated areas shall be subject to a warranty period of not less than 12 months from initial establishment of vegetation over 100 percent of the areas seeded.
- B. At the end of the warranty period, the Construction Manager will perform an inspection of the area. Seeded areas not demonstrating satisfactory condition of vegetation as specified in this Section, shall be repaired, reseeded, and maintained to meet requirements as specified in this Section at the Contractor's expense.
- C. Areas that fail to meet these requirements shall be repaired or reseeded as necessary to produce an acceptable stand of vegetation, as specified in this Section. For the OSDF Cell Final Cover vegetation only, areas that fail to meet these requirements shall be repaired as necessary and reseeded to produce an acceptable stand of vegetation by using an alternate seed mix such as hydroseeding tall fescue as determined to be appropriate by the Construction Manager.

3.06 ACCEPTANCE

- A. The vegetated areas shall be accepted at the end of the warranty period if a satisfactory condition exists as defined in this Section.
- B. After disturbed areas are stabilized and all necessary corrective work has been completed, the Construction Manager will certify in writing the final acceptance of the vegetated areas.

3.07 CONSTRUCTION QUALITY REQUIREMENTS

- A. CQC Consultant will monitor vegetation and crusting agent application in accordance with this Section and Construction Quality Assurance (CQA) Plan.

TABLE 02930-1A

SEED MIX IN DRY AREAS FOR PERMANENT VEGETATION

SPECIES	POUNDS PER ACRE
	(lb/ac)
Big Bluestem (<i>Andropogon gerardi</i>)	3
Little Bluestem (<i>Andropogon scoparius</i>)	2
Side-Oats Grama (<i>Bouteloua curtipendula</i>)	0.5
Indian Grass (<i>Sorghastrum nutans</i>)	2
Canada Wild-Rye (<i>Elymus canadensis</i>)	25
Switch grass (<i>Panicum virgatum</i>)	0.5
ReGreen	5
Wildflowers ⁽¹⁾ :	1.5
Butterflyweed (<i>Asclepias tuberosa</i>)	
New England Aster (<i>Aster novae-angliae</i>)	
Smooth Aster (<i>Aster laevis</i>)	
Canada Milkvetch (<i>Astragalus Canadensis</i>)	
Purple Prairie Clover (<i>Petalostemum purpureum</i>)	
Ox-eye Sunflower (<i>Heliopsis helianthoides</i>)	
Bergamot (<i>Monarda fistulosa</i>)	
Purple Coneflower (<i>Echinacea purpurea</i>)	
Pale Purple Coneflower (<i>Echinacea pallida</i>)	
Yellow Coneflower (<i>Ratibida pinnata</i>)	
Black-Eyed Susan (<i>Rudbeckia hirta</i>)	
Spiderwort (<i>Tradescantia ohioensis</i>)	
Blue Vervain (<i>Verbena hastata</i>)	
Hoary Vervain (<i>Verbena stricta</i>)	
Beardtongue (<i>Penstemon grandiflorus</i>)	
Cupplant (<i>Silphium perfoliatum</i>)	
Sweet Joe Pye-Weed (<i>Eupatorium purpureum</i>)	
White False Indigo (<i>Baptisia leucantha</i>)	
Blue False Indigo (<i>Baptisia australis</i>)	
Partridge Pea (<i>Cassia fasciculata</i>)	
Rattlesnake Master (<i>Eryngium yuccifolium</i>)	
Round-headed Bush Clover (<i>Lespedeza Capitata</i>)	
Stiff Goldenrod (<i>Solidago rigida</i>)	

Note: (1) Wildflower mix to be apportioned according to species aggressiveness and seed counts as approved by the Construction Manager. If certain species are not available, appropriate substitutions will be approved by the Construction Manager.

TABLE 02930-1B

SEED MIX IN WET AREAS⁽¹⁾ FOR PERMANENT VEGETATION

Species	POUNDS PER ACRE (lb/ac)
Big Bluestem (<i>Andropogon gerardi</i>)	3
Canada Wild-Rye (<i>Elymus canadensis</i>)	25
S Grass (<i>Panicum virgatum</i>)	0.5
Blue Joint Grass (<i>Calamagrostis canadensis</i>)	0.5
Porcupine Sedge (<i>Carex hystericina</i>)	1 ounce per acre (oz/ac)
Fox Sedge (<i>Carex stipata</i>)	1 ounce per acre (oz/ac)
Dark Green Bulrush (<i>Scirpus atrovirens</i>)	1 ounce per acre (oz/ac)
ReGreen	5
Prairie Cordgrass (<i>Spartina pectinata</i>)	1
Wildflowers ⁽²⁾ :	1.5
Red Milkweed (<i>Asclepias incarnata</i>)	
New England Aster (<i>Aster novae-angliae</i>)	
Wild Senna (<i>Cassia hebecarpa</i>)	
Canada Tick Trefoil (<i>Desmodium canadense</i>)	
Prairie Blazingstar (<i>Liatris pycnostachya</i>)	
Great Blue Lobelia (<i>Lobelia siphilitica</i>)	
Bergamot (<i>Monarda fistulosa</i>)	
Yellow Coneflower (<i>Ratibida pinnata</i>)	
Branched Coneflower (<i>Rudbeckia hirta</i>)	
Blue Vervain (<i>Verbena hastata</i>)	
Angelica (<i>Angelica atropurpurea</i>)	
Sweet Joe-Rye Weed (<i>Eupatorium purpureum</i>)	

Notes: (1) Seeding in drainage ditches or swales shall contain erosion mats as specified in Section 02270 after application of seed mixture. Erosion mat shall cover a minimum width of 12 feet.

(2) Wildflower mix to be apportioned according to species aggressiveness and seed counts as approved by the Construction Manager. If certain species are not available, appropriate substitutions will be approved by the Construction Manager.

TABLE 02930-1C

SEED MIX FOR CELL FINAL COVER PERMANENT VEGETATION

Species	POUNDS PER ACRE (lb/ac)
Grass	
Big Bluestem (<i>Andropogon gerardi</i>)	0.5
Little Bluestem (<i>Andropogon scoparius</i>)	3
Side-Oats Grama (<i>Bouteloua curtipendula</i>)	5
Buffalo Grass (<i>Buchloe dactyloides</i>)	1
Indian Grass (<i>Sorghastrum nutans</i>)	0.5
Canada Wild-Rye (<i>Elymus Canadensis</i>)	25
Annual Rye (<i>Lolium multiflorum</i>)	10
Prarie Dropseed (<i>Sporobulus heterolepis</i>)	1.5
Species	Ounces Per Acre (oz/ac)
Wildflower	
Butterflyweed (<i>Asclepias tuberosa</i>)	3.125
Smooth Aster (<i>Aster laevis</i>)	0.25
Ox-eye Sunflower (<i>Heliopsis helianthoides</i>)	1.75
Bergamot (<i>Monarda fistulosa</i>)	0.25
Purple Coneflower (<i>Echinacea purpurea</i>)	2.0
Pale Purple Coneflower (<i>Echinacea pallida</i>)	2.0
Yellow Coneflower (<i>Ratibida pinnata</i>)	0.375
Black-Eyed Susan (<i>Rudbeckia hirta</i>)	1.0
Spiderwort (<i>Tradescantia ohioensis</i>)	1.25
Hoary Vervain (<i>Verbena stricta</i>)	0.50
Beardtongue (<i>Penstemon grandiflorus</i>)	1.0
Sweet Joe Pye-Weed (<i>Eupatorium perpureum</i>)	0.25
White False Indigo (<i>Baptisia leucantha</i>)	4.25
Blue False Indigo (<i>Baptisia australis</i>)	4.25
Partridge Pea (<i>Cassia fasciculata</i>)	32
Round-headed Bush Clover (<i>Lespedeza Capitata</i>)	1.0
Stiff Goldenrod (<i>Solidago rigida</i>)	0.75

TABLE 02930-2
SEED MIX FOR INTERIM VEGETATION

Species	Pounds Per Acre (lb/ac)
ReGreen	50
Partidge Pea (<i>Cassia fasciculate</i>)	10
Canada Wild Rye (<i>Elymus Canadensis</i>)	40

[END OF SECTION]